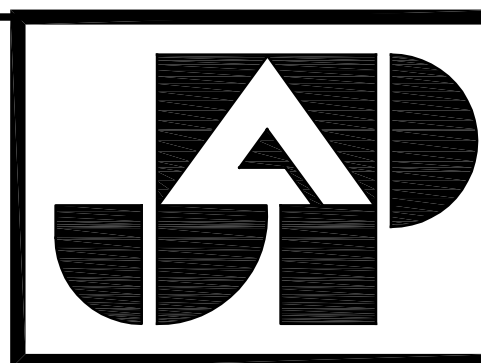
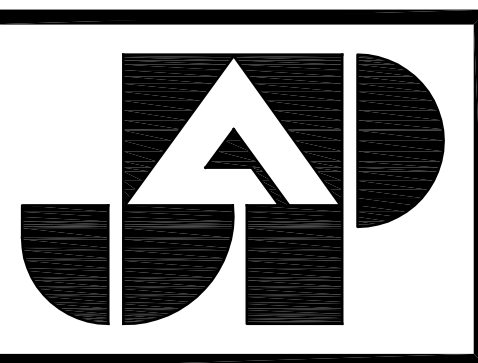




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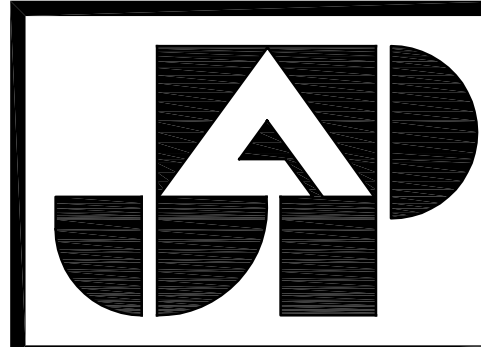
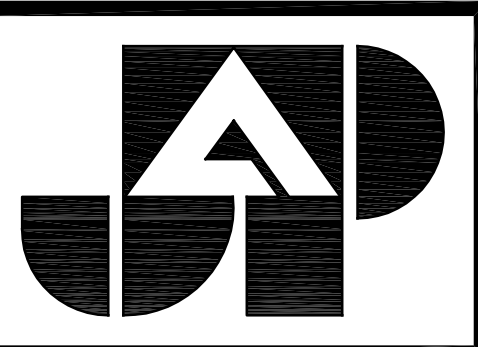
JANUARY 23, 2015

LIST OF STRUCTURAL DRAWINGS

Drawing Index		Issue Date					
		ISSUED FOR PERMIT	REISSUED FOR PERMIT	ISSUED FOR TENDER	ISSUED FOR CONSTRUCTION REVIEW	RE ISSUED FOR BUILDING PERMIT	
		MAY 30, 2014	JULY 10, 2014	AUG 27, 2014	DEC 15, 2014	JAN 23, 2015	
GENERAL NOTES AND TYPICAL DETAILS							
S-001	GENERAL NOTES & TYPICAL DETAILS	X	X	X	X	X	
S-002	TYPICAL DETAILS	X	X	X	X	X	
S-003	TYPICAL DETAILS	X	X	X	X	X	
S-004	TYPICAL DETAILS	X	X	X	X	X	
S-005	TYPICAL DETAILS	X	X	X	X	X	
PLANS							
S-101	P2 PARKING LEVEL (FOUNDATION PLAN)	X	X	X	X	X	
S-102	P1 PARKING LEVEL FRAMING PLAN	X	X	X	X	X	
S-103	GROUND FLOOR FRAMING PLAN	X	X	X	X	X	
S-104	2ND FLOOR FRAMING PLAN	X	X	X	X	X	
S-105	3RD FLOOR FRAMING PLAN	X	X	X	X	X	
S-106	4TH FLOOR FRAMING PLAN	X	X	X	X	X	
S-107	5TH FLOOR FRAMING PLAN	X	X	X	X	X	
S-108	6TH FLOOR FRAMING PLAN	X	X	X	X	X	
S-109	7TH FLOOR FRAMING PLAN	X	X	X	X	X	
S-110	8TH FLOOR FRAMING PLAN	X	X	X	X	X	
S-111	9TH FLOOR FRAMING PLAN	X	X	X	X	X	
S-112	10TH FLOOR FRAMING PLAN	X	X	X	X	X	
S-113	11TH FLOOR FRAMING PLAN	X	X	X	X	X	
S-114	12TH FLOOR FRAMING PLAN	X	X	X	X	X	
S-115	13TH FLOOR FRAMING PLAN	X	X	X	X	X	
S-116	14TH FLOOR FRAMING PLAN	X	X	X	X	X	
S-117	PENTHOUSE AND UPPER ROOF FLOOR FRAMING PLAN	X	X	X	X	X	
COLUMN AND WALL SCHEDULE							
S-201	COLUMN AND WALL SCHEDULE (BELOW GRD. FLOOR)	X	X	X	X	X	
S-202	COLUMN AND WALL SCHEDULE	X	X	X	X	X	
S-203	COLUMN AND WALL SCHEDULE	X	X	X	X	X	
S-204	COLUMN AND WALL SCHEDULE	X	X	X	X	X	
S-205	COLUMN AND WALL SCHEDULE	X	X	X	X	X	
S-206	COLUMN AND WALL SCHEDULE			X	X	X	
SECTIONS							
S-301	FOUNDATION SECTIONS	X	X	X	X	X	
S-302	FOUNDATION SECTIONS	X	X	X	X	X	
S-303	FOUNDATION SECTIONS	X	X	X	X	X	
S-304	FOUNDATION SECTIONS	X	X	X	X	X	
S-305	SECTION & DETAILS					X	



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1. ALL DIMENSIONS SHOWN ON STRUCTURAL DRAWINGS SHALL BE VERIFIED WITH ARCHITECTURAL DRAWINGS AND ANY DISCREPANCIES SHALL BE REPORTED TO THE ARCHITECT IMMEDIATELY.
2. READ STRUCTURAL DRAWINGS IN CONJUNCTION WITH SPECIFICATIONS AND ALL RELATED CONTRACT DOCUMENTS, WHICH SHALL BE THE MOST CURRENT DOCUMENTS, STANDARDS AND CODES ISSUED AT THE TIME OF THE CONTRACT.
3. REFER TO ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR SIZES AND LOCATIONS OF SLEEVES NOT SHOWN ON THE STRUCTURAL DRAWINGS.
4. REFER TO ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR DETAILS RELATING TO TRENCHES, SUMP PITS, HOUSEKEEPING PADS, BASES, CURBS AND THE LIKE, NOT DELINEATED ON THE STRUCTURAL DRAWINGS.
5. ALL TRADES SHALL WORK IN CONFORMANCE WITH THE SAFETY REGULATIONS OF THE ONTARIO MINISTRY OF LABOUR AND EMPLOYMENT.
6. THE STRUCTURAL DESIGN LOADS SHOWN ON THE DRAWINGS SHALL NOT BE EXCEEDED DURING CONSTRUCTION.

- CAISSONS SHALL BE CARRIED DOWN TO, AND BEAR ON NATURAL UNDISTURBED SOUND BEARING MATERIAL, AS DESCRIBED IN THE SOIL REPORT, WITH A MINIMUM SAFE BEARING CAPACITY AS INDICATED ON FOUNDATION PLANS.
2. EXTERIOR FOOTINGS SHALL BE FOUND AT A MINIMUM OF 1200mm BELOW FINISHED GRADE. FOOTINGS EXPOSED TO EXISTING GRADE SHALL BE PROTECTED BY AN EXISTING CURB OR OTHER PROTECTIVE MEASURES.
3. NEW FOOTINGS WHICH ARE ADJACENT TO EXISTING FOOTINGS SHALL BE FOUND AT THE SAME ELEVATION AS THE EXISTING FOOTINGS UNLESS OTHERWISE SHOWN OR NOTED.
4. THE LINE OF SLOPE BETWEEN ADJACENT EXCAVATIONS FOR FOOTINGS OR ALONG STEEPED FOOTINGS SHALL NOT EXCEED A 1:1 RATIO OF 1:1. MAXIMUM SLOPE IN A STEEPED FOOTING SHALL BE 600mm.
5. CAP DIMENSIONS GIVEN IN THE SCHEDULE, ARE BASED ON ASSUMED DESIGN CONDITIONS. THE ACTUAL SOIL CONDITIONS ARE TO BE FOUND IN THE FIELD WITH THOSE ASSUMED. FOOTING BASE ELEVATION SHALL BE ADJUSTED AND THE DEPTH OF THE CAPS SHALL BE MODIFIED IN ACCORDANCE WITH THE FOLLOWING LIMITING CRITERIA:
 - a) MINIMUM DEPTH OF THE CAPS SHALL BE A CONCRETE SLAB WITH A MINIMUM OF 150mm.
 - b) (1) TWICE THE GREATEST HORIZONTAL PROJECTION OF THE CAP BEYOND THE COLUMN FACE, AND
 - c) (2) TWICE THE LENGTH OF THE COLUMN DIMENSIONS, PLUS 300mm, MINUS THE DEPTH OF THE FOOTING BASE.
6. MINIMUM DEPTH OF CAP UNDER A STRUCTURAL STEEL COLUMN SHALL BE TWICE THE GREATEST HORIZONTAL PROJECTION OF THE CAP BEYOND THE EDGE OF THE COLUMN BASE PLATE.
7. MINIMUM DEPTH OF CAP UNDER A CONCRETE COLUMN SHALL BE 3 TIMES ITS DIAMETER, UNLESS REINFORCED.
8. WHERE THERE IS GRADE ON BOTH SIDES OF A FOUNDATION WALL, THE BACKFILL AGAINST THE WALL SHALL BE GARRIED OUT SO THAT THE DIFFERENCE IN THE DEPTH OF THE BACKFILL ON OPPOSITE SIDES OF THE WALL IS NEVER MORE THAN 150mm.
9. DO NOT PLACE BACKFILL AGAINST WALLS, OTHER THAN CANTILEVERED RETAINING WALLS, UNTIL THE WALLS AND THE FLOOR SLABS, AT THE TOP AND BOTTOM OF THE WALLS HAVE DEVELOPED THEIR FULL SPECIFIED CONCRETE DESIGN STRENGTH.
10. WHERE A SLAB-ON-GRADE IS EMPLOYED TO RESTRAIN THE TOP OF A WALL, SUBJECTED TO LATERAL EARTH PRESSURES THE WALL SHALL BE ADEQUATELY TEMPORARILY SHORED UNTIL THE CONCRETE IN THE SLAB-ON-GRADE HAS DEVELOPED ITS FULL DESIGN STRENGTH.
11. SLABS ON EARTH SHALL BE CAST IN ALTERNATE STRIPS NOT EXCEEDING 4500mm IN WIDTH BETWEEN KEVED CONSTRUCTION JOINTS. THE CAST STRIPS SHALL BE SAW-CUT INTO PANELS NOT EXCEEDING 4500mm IN LENGTH.
12. THE MAXIMUM LENGTH OF A WALL CASTING SHALL BE 18000mm BETWEEN KEVED CONSTRUCTION JOINTS. CHASES AND SLOTTING SHALL BE CAST IN THE BEAMS SHOWN IN THE SCHEDULE.
13. CRUSHED STONE BASES BELOW SLABS-ON-GRADE SHALL BE COMPACTED TO THE DEGREE RECOMMENDED IN THE SOIL REPORT FOR THIS PROJECT.
14. CONTRACTOR SHALL CONTROL SURFACE AND GROUND WATER DURING CONSTRUCTION TO ENABLE FOUNDATIONS TO BE CONSTRUCTED ON DRY SOIL AND ROCK.
15. NO FOUNDATIONS SHALL BE CAST ON FROZEN MATERIAL OR IN WATER.

CONCRETE

1. CONCRETE AND CONCRETE WORK SHALL CONFORM TO CSA-A23.1-09/A23.2-09, CAN/CSA-S413-07 AND TO THE PROPERTIES FOR STRENGTH AND EXPOSURE CLASSIFICATION PRESENTED IN THE NOTES FOR THE RELEVANT PLANS AND SCHEDULES.
2. REINFORCING STEEL SHALL CONFORM TO CAN/CSA-G30.18-M92 (R2007), AND SHALL HAVE A YIELD STRENGTH OF 400 MPa UNLESS OTHERWISE NOTED.

1. STRUCTURAL STEEL, WIDE FLANGE AND WELDED WIDE FLANGE SHAPES SHALL CONFORM TO CAN/CSA-G40.21-04/GR40.21-04, GRADE 350W, UNLESS OTHERWISE NOTED.
2. STRUCTURAL STEEL, CHANNEL SHAPES SHALL CONFORM TO CAN/CSA-G40.21-04/GR40.21-04, GRADE 300W/LS, UNLESS OTHERWISE NOTED.
3. STRUCTURAL STEEL, HOLLOW TUBES SHALL CONFORM TO CAN/CSA-G40.21-04/GR40.21-04, GRADE 350W, CLASS H.
4. STRUCTURAL BOLTS, NUTS AND WASHERS SHALL CONFORM TO ASTM A325M.
5. SECONDARY BOLTS IN GIRTS, LIGHT ANGLE FRAMES AND ANCHOR BOLTS SHALL CONFORM TO ASTM A307, UNLESS OTHERWISE NOTED.
6. CONCRETE EMBEDDED STEEL STUDS SHALL BE MANUFACTURED OF LOW CARBON STEEL, GRADE 300W, THAT SHALL BE WELDED IN CONFORMANCE WITH AWS D1.1/01:1M-2004, CSA-W99-03 AND W818-M 1990 (P2007). CONCRETE EMBEDDED STEEL RAILS SHALL BE MANUFACTURED OF LOW CARBON STEEL, GRADE 300W, AND SHALL RUSTOUT THE FULL YIELD

1. CONCRETE MASONRY BLOCK UNITS SHALL CONFORM TO CSA-A165-04, WITH A COMPRESSIVE STRENGTH OF 15 MPa, BASED ON NET AREA, UNLESS OTHERWISE NOTED.
2. MORTAR SHALL CONFORM TO CSA-A179-04, TYPE 'N' TYPICALLY AND TYPE 'S' BELOW GRADE, UNLESS OTHERWISE NOTED.
3. NON-SHRINK GROUT SHALL CONFORM TO CSA-A231-09/A232-09, AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 35 MPa AT 28 DAYS.
4. MASONRY GROUT SHALL CONFORM TO CSA-A179-04, AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 20 MPa AT 28 DAYS, AND A MAXIMUM SLUMP OF 250mm, UNLESS OTHERWISE NOTED.

1. REINFORCING STEEL SHALL BE SECURELY HELD IN PLACE DURING THE CASTING AND COMPACTING OF THE CONCRETE. IF REQUIRED, ADDITIONAL SUPPORT BARS AND/OR STIRRUPS SHALL BE PLACED BY THE CONTRACTOR TO PROVIDE SUPPORT FOR ALL TOP BARS IN SLABS SHALL BE SUPPORTED ON CONTINUOUS HOOT CHAIRS.
2. CONTINUOUS REINFORCING BARS SHALL BE LAPPED AT SPLICES AND CORNERS IN CONFORMANCE WITH CSA-A23-04 AND THE TYPICAL DETAILS, UNLESS OTHERWISE NOTED. LAP CONTINUOUS TOP BARS AT CENTER BETWEEN SUPPORTS. BOTTOM BARS AT SUPPORTS. WHERE REQUIRED, TERMINATE CONTINUOUS BARS AT NON-CONTINUOUS ENDS WITH STANDARD HOOKS.
3. PROVIDE DOWELS TO WALLS AND COLUMNS WHICH SHALL BE THE SAME IN NUMBER, SIZE AND SPACING, AS THE VERT REINFORCING STEEL IN THE WALLS OR COLUMNS ABOVE, UNLESS OTHERWISE NOTED.
4. ALL DOWELS SHALL HAVE A MINIMUM EMBEDMENT LENGTH EQUAL TO THE STRAIGHT TENSION EMBEDMENT LENGTH FOR THE SIZE OF THE BARS. DOWELS FROM WALLS TO SLABS SHALL HAVE A MINIMUM EMBEDMENT LENGTH OF 600mm into concrete and 50mm into slabs, UNLESS OTHERWISE INDICATED.
5. CONCRETE BEAMS AND SPANDRELS SHALL BE CAST MONOLITHICALLY WITH SLABS UNLESS OTHERWISE DELINEATED. HORIZONTAL CONSTRUCTION JOINTS SHALL NOT BE MADE IN BEAMS OR SPANDRELS UNLESS APPROVED BY THE CONSULTANT. BEAMS AND SPANDRELS SHALL BE REINFORCED WITH TOP AND BOTTOM BARS AT MID-SPANS OF BEAMS, SPANDRELS AND SLABS, WHICH SHALL BE REVIEWED AND APPROVED BY THE CONSULTANT.
6. THE LOCATION AND DETAILS OF CONSTRUCTION JOINTS REQUIRED, BUT NOT SHOWN, SHALL BE SUBMITTED TO THE CONSULTANT FOR APPROVAL.
7. NO OPENINGS SHALL BE FORMED IN THE CONCRETE STRIPS OF FLAT PLATE OR FLAT SLAB CONSTRUCTION WITHOUT THE CONSULTANT'S APPROVAL.
8. NO SLEEVES SHALL BE PLACED IN BEAMS OR THROUGH COLUMNS WITHOUT THE CONSULTANT'S APPROVAL.
9. MINIMUM CONCRETE COVER TO REINFORCING BARS IN A NON-CORROSIVE SITUATION SHALL BE 75mm FOR CONCRETE CAST AGAINST AND EXPOSED TO SOIL.
10. MINIMUM CONCRETE COVER TO REINFORCING BARS IN A NON-CORROSIVE SITUATION FOR CONCRETE CAST ON OR WITHIN FORMWORK SHALL BE AS FOLLOWS IN mm:

ELEMENT	REINFORCING BAR DIAMETER (mm)	NOT EXPOSED TO EARTH OR WEATHER (COVER mm)	EXPOSED TO EARTH OR WEATHER (COVER mm)
SLABS AND WALLS	20 AND SMALLER	25	30
	25	25	40
	30	30	45
	35	35	55
	45	45	70
COLUMNS (AXIAL REINFORCEMENT)	55	55	85
	35 AND SMALLER	50	55
	45	50	70
BEAMS, GIRDERS AND PILES (PRINCIPAL REINFORCEMENT)	55	55	85
	35 AND SMALLER	40	55
	45	45	70
TIES, STIRRUPS AND SPIRALS	55	55	85
	-	30	40

N.B. MINIMUM CONCRETE COVER TO PRINCIPAL REINFORCING IN SHEAR WALL = 40mm

11. MINIMUM CONCRETE COVER TO REINFORCING BARS FOR A 3 HOURS FIRE RATING SHALL BE 35mm FOR BOTTOM BARS IN SLAB, 65mm FOR MAX AXIAL REINFORCING IN COLUMNS, 35mm FOR WALLS and 40mm in BEAMS FOR BOTTOM BARS AND SMALLER, AND 45mm AND 55mm FOR 45M AND 50M BARS RESPECTIVELY.
12. MINIMUM CONCRETE COVER TO REINFORCING BARS FOR CONCRETE EXPOSED TO DE-ICING CHEMICALS, ON PARKING GARAGE SLABS, DAMPS AND TRUCK DOCKS SHALL BE 40mm, IN GARAGE WALLS AND TOP REINFORCING BARS IN SLAB AND 30mm FOR BOTTOM REINFORCING BARS.
13. UNLESS OTHERWISE SPECIFIED ON CAMBER DRAWINGS OF FRAMING LEVELS, CAMBER ALL SLAB AND BEAM FORMS 5mm FOR EACH 3000mm OF SPAN. THE CAMBER SHALL BE CHECKED AND ADJUSTED BEFORE THE INITIAL SET OF CONCRETE. THE 12mm TO 15mm MINIMUM CAMBER SHALL BE MAINTAINED THROUGHOUT THE FULL THICKNESS OF SLABS AND BEAMS SH BE MAINTAINED THROUGHOUT. IN PARKING LEVELS, CAMBER SHALL BE INCREASED AS MAY BE REQUIRED TO ENSURE A MINIMUM NOMINAL SLOPE OF 2 PERCENT TO DRAINS AFTER ALL LONG TERM DEFLECTIONS OCCUR.
14. UNLESS SPECIFIED OTHERWISE, RESHOERING SHALL BE INSTALLED FOR A MINIMUM OF 3 FLOORS BELOW THE LATEST SLAB CASTING.
15. LAP SPICES FOR REINFORCING STEEL SHALL CONFORM TO CSA-A23.3-04, TAKING INTO ACCOUNT THE CONCRETE STRENGTH, GRADE OF STEEL, TOP OR BOTTOM BARS AND COATINGS ON STEEL, ANY.
16. ELECTRICAL CONDUITS AND MECHANICAL PIPES ARE NOT PERMITTED IN CONCRETE COLUMNS UNLESS PREVIOUSLY APPROVED BY THE STRUCTURAL CONSULTANT.
17. ONLY VERTICAL CONDUITS OR PIPE ARE PERMITTED IN CONCRETE WALLS.
18. WHERE CONDITIONS DISPLACE MORE THAN 5% OF THE CROSS SECTIONAL AREA OF THE SLAB, CONCRETE WITH 10mm AGGREGATE (PEA GRAVEL) SHALL BE USED AND THE CONSULTANT SHALL BE NOTIFIED. IF THE CONCRETE IS DEEMED TO BE SIGNIFICANT THE CONSULTANT RESERVES THE RIGHT TO INCREASE THE COVER TO THE REINFORCEMENT.
19. MAXIMUM OUTSIDE DIAMETER OF EMBEDDED PIPES AND CONDUITS SHALL BE 50mm. CONDUITS SHALL BE SPACED A MINIMUM OF 3 DIAMETERS APART.

1. STRUCTURAL STEEL WORK SHALL CONFORM TO CAN/CSA-516.
2. STEEL BEAM PLATES SHALL HAVE A MINIMUM BEARING LENGTH OF 200mm ON MASONRY OR CONCRETE, UNLESS OTHERWISE NOTED.
3. STEEL BEARING PLATES SHALL BE CENTERED UNDER THE BEAM WEBS, UNLESS NOTED OTHERWISE.
4. BEAM BEARING PLATES DIMENSION GIVEN FIRST IS PARALLEL TO THE SPAN OF THE BEAM.
5. WHERE MOMENT CONNECTIONS ARE INDICATED, THE CONNECTION SHALL DEVELOP THE FULL FLEXURAL CAPACITY OF THE SMALLER CONNECTED MEMBER.
6. MEMBER SPLICES SHALL DEVELOP THE FULL FLEXURAL AND SHEAR CAPACITY OF THE MEMBER. MEMBERS SHALL NOT BE SPliced AT THE SAME MEMBER CROSS SECTION. SPLICES SHALL BE MADE ONLY WITH THE APPROVAL OF THE CONSULTANT.
7. INSTALL FITTED, WELDED STIFFENER PLATES AT EACH SIDE OF BEAM WEB AT LOCATIONS OF CONCENTRATED LOADS, SUCH AS A SUPPORT FOR A COLUMN ABOVE, OR OVER A COLUMN, UNLESS OTHERWISE SHOWN.
8. COLUMNS SHALL BE CONNECTED TO BASE PLATES FOR THE GREATER OF THE FOLLOWING, PLUS OTHER FORCES SHOWN, UNLESS OTHERWISE NOTED:
 - a. 80 PERCENT OF THE VERTICAL COLUMN LOAD APPLIED HORIZONTALLY.
 - b. THE RESULTANT OF PULLING FORCES.
9. FABRICATE GABLES TO PURCHASING BEAMS, GIRDERS AND TRUSSES, AS SHOWN ON THE FRAMING PLANS. NOTED MEMBER CAMBERS ARE FOR THE ERECTED CONFIGURATION BEFORE STEEL DECK IS INSTALLED.
10. INSTALL WELDED L-75x56x6 BRACKET, AT 45 DEGREES TO THE BEAM AXIS, WHERE BEAMS CONNECT AT COLUMNS, TO SUPPORT STEEL DECK, WHERE THE COLUMN CONTAINS ABOVE THE LEVEL OF THE DECK.
11. INSTALL AND MAINTAIN TEMPORARY BRACING UNTIL THE ENTIRE STRUCTURAL STEEL FRAME IS COMPLETED, INCLUDING FLOOR AND ROOF DECKS AND ALL ELEMENTS WHICH ARE PART OF THE LATERAL LOAD RESISTING SYSTEM.
12. ALL STRUCTURAL STEEL EXPOSED TO THE ELEMENTS SHALL BE HOT-DIPPED GALVANIZED UNLESS OTHERWISE NOTED. WHERE AN ANTICORROSION COATING HAS BEEN COMPROMISED, TOUCH-UP WITH ZINC RICH PRIMER AS APPROVED BY THE CONSULTANT.

1. MASONRY CONSTRUCTION SHALL CONFORM TO CSA-A371-04.
2. BELOW BEARING PLATES OF CONCRETE AND STEEL BEAMS, JOISTS AND TRUSSES, PROVIDE SOLID MASONRY BEARING PAD OF ONE HUNDRED PERCENT SOLID MASONRY UNITS, LAID IN TYPE "S" MORTAR. THE BEARING PAD LENGTH SHALL BE EQUAL TO THE BEARING LENGTH OF THE BEARING PLATE AND SHALL EXTEND DOWN BELOW THE BEARING PLATE, TO A DEPTH EQUAL TO THE LENGTH OF THE BEARING PLATE.
3. CONCRETE SLABS BEARING ON MASONRY SHALL HAVE A MINIMUM BEARING OF 200mm and THE BEARING COURSE SHALL BE FULL HEIGHT OF THE BEARING COURSE. ALL MASONRY UNITS LAID IN TYPE "S" MORTAR EXTENDING DOWN TO A DEPTH OF 200mm BELOW THE UNDERSIDE OF THE SLAB.
4. AT THE BEARING ENDS OF STEEL, CONCRETE OR REINFORCED CONCRETE BLOCK UNITS, PROVIDE 200mm LONG BY 200mm DEEP BEARING PADS OF ONE HUNDRED PERCENT SOLID MASONRY UNITS LAID IN TYPE "S" MORTAR.
5. WHERE STEEL BEAMS FRAME INTO MASONRY WALLS, AND STEEL COLUMNS ARE EMBEDDED IN MASONRY WALLS, CONSTRUCT MASONRY WALLS TO TIGHTLY FIT THE SPACES BETWEEN THE FLANGES AND TO BE IN FULL CONTACT WITH BOTH SIDES OF THE WEBS.
6. OVER ALL OPENINGS AND RECESSES IN MASONRY WALLS, UNLESS OTHERWISE NOTED, PROVIDE 200mm DEEP UNIFORM CORNER REINFORCEMENT BARS TO BE LOCATED AS SHOWN IN THE DETAILS. PROVIDE 100% TOP AND BOTTOM CORNER REINFORCEMENT AS FOLLOWS:
 - a. CLEAR SPANS UP TO 1200mm USE 1-10M, HOOKED EACH END, TOP AND BOTTOM, FOR EACH 90mm OF WALL THICKNESS OR PORTION THEREOF.
 - b. CLEAR SPANS FROM 1200mm TO 1800mm USE 1-15M, HOOKED EACH END, TOP AND BOTTOM, FOR EACH 90mm OF WALL THICKNESS OR PORTION THEREOF.

FOR BRICK WALLS, USE ONE L-90x90x8 FOR CLEAR SPANS UP TO 1200mm FOR EACH 90mm OF WALL THICKNESS, AND FOR CLEAR SPANS BETWEEN 1200mm AND 1800mm, USE ONE L-125x90x8 (LLV) FOR EACH 90mm OF WALL THICKNESS.

7. REFER TO LINTEL SCHEDULES FOR FURTHER DETAILS.

8. WALL THICKNESS CHANGES SHALL INCORPORATE 100% SOLID MASONRY UNITS FOR A HEIGHT OF 200mm BELOW THE REDUCTION IN WALL THICKNESS.

1. LIVE SERVICE LOADS ARE AS FOLLOWS, UNLESS OTHERWISE NOTED ON PLANS :	
LIVING QUARTERS, UPPER FLOOR CORRIDORS _____	1.9 kPa
ENTRANCE HALLS, GROUND FLOOR CORRIDORS, EXITS, STAIRS, STORAGE AREAS, RECREATION AREAS _____	4.8 kPa
BALCONIES, SUNROOMS & LOCKER ROOMS _____	4.8 kPa
GARAGES FOR PASSENGER VEHICLES _____	2.4 kPa
LOADING DOCKS AND RAMPS FOR BULK LIFT VEHICLES _____	14.0 kPa
MECHANICAL ROOMS _____	7.2 kPa
RETAIL AREAS _____	4.8 kPa
SIDEWALKS AND DRIVEWAYS OVER AREAWAYS AND BASEMENTS _____	12.0 kPa

1. THE STRUCTURE HAS BEEN DESIGNED TO RESIST WIND PRESSURE AND LIVE LOAD DUE TO EARTHQUAKE IN ACCORDANCE WITH ONTARIO BUILDING CODE 2012, AND THE REFERENCED CLIMATIC INFORMATION FOR TORONTO, ONTARIO.

2. LIVE LOADS DUE TO SNOW AND RAIN.
Se=0.9 kPa
Sra=0.4 kPa

3. LIVE LOADS DUE TO WIND FOR TORONTO, ONTARIO.
IMPORTANCE FACTOR $I_w = 1.0$
HOURLY WIND PRESSURES: $(1/10) = 0.34$ kPa
 $(1/50) = 0.44$ kPa
E-W, Vw = 4397 kN, Mw = 109610 kN.m
N-S, Vw = 2082 kN, Mw = 60308 kN.m

4. LIVE LOADS DUE TO EARTHQUAKE FOR TORONTO, ONTARIO.
IMPORTANCE FACTOR $I_e = 1.0$
Se(0.2) = 0.22, Sa(0.5) = 0.13, Se(1.0) = 0.067, Se(2.0) = 0.021
SITE CLASS = C, Fa = 1.0, Fv = 1.0
IefSS(0.2) = 0.22
TYPE OF IRREGULARITIES = NONE
METHOD OF ANALYSIS = STATIC
To = 0.851 Mx = 1.0 Jw = 1.0
TYPE OF SFRS = CONVENTIONAL SHEAR WALLS Rd = 1.5 Rom = 1.3
N-S Vw = 9669 kN, Mw = 287107 kN.m
E-W, Vw = 10856N, Mw = 323264 kN.m

107

TABLE 5

SLABS AND BEAMS OTHER THAN TOP BARS

LAP SPlice LENGTHS AND DEVELOPMENT LENGTHS

DATA	BAR SIZE	10	15	20	25	30	35
TENSION $f_y = 400 \text{ MPa}$ $f_c = 20 \text{ MPa}$	LAP SPlice	420	630	840	1050	1260	1470
	DEVELOPMENT LENGTH	330	495	650	810	970	1130
	LAP SPlice	300	440	580	730	880	1030
COMPRESSION $f_y = 400 \text{ MPa}$ $f_c = 20 \text{ MPa}$	LAP SPlice	220	330	430	540	640	750
	DEVELOPMENT LENGTH	200	300	400	500	600	700
	LAP SPlice	200	290	380	480	580	680
TENSION $f_y = 450 \text{ MPa}$ $f_c = 25 \text{ MPa}$	LAP SPlice	390	570	750	930	1110	1290
	DEVELOPMENT LENGTH	300	440	580	730	880	1030
	LAP SPlice	300	440	580	730	880	1030
COMPRESSION $f_y = 450 \text{ MPa}$ $f_c = 25 \text{ MPa}$	LAP SPlice	200	290	380	480	580	680
	DEVELOPMENT LENGTH	200	280	360	450	540	630
	LAP SPlice	200	280	360	450	540	630
TENSION $f_y = 500 \text{ MPa}$ $f_c = 30 \text{ MPa}$	LAP SPlice	390	570	750	930	1110	1290
	DEVELOPMENT LENGTH	300	440	580	730	880	1030
	LAP SPlice	300	440	580	730	880	1030
COMPRESSION $f_y = 500 \text{ MPa}$ $f_c = 30 \text{ MPa}$	LAP SPlice	200	290	380	480	580	680
	DEVELOPMENT LENGTH	200	280	360	450	540	630
	LAP SPlice	200	280	360	450	540	630
TENSION $f_y = 550 \text{ MPa}$ $f_c = 35 \text{ MPa}$	LAP SPlice	390	570	750	930	1110	1290
	DEVELOPMENT LENGTH	300	440	580	730	880	1030
	LAP SPlice	300	440	580	730	880	1030
COMPRESSION $f_y = 550 \text{ MPa}$ $f_c = 35 \text{ MPa}$	LAP SPlice	200	290	380	480	580	680
	DEVELOPMENT LENGTH	200	280	360	450	540	630
	LAP SPlice	200	280	360	450	540	630
TENSION $f_y = 600 \text{ MPa}$ $f_c = 40 \text{ MPa}$	LAP SPlice	390	570	750	930	1110	1290
	DEVELOPMENT LENGTH	300	440	580	730	880	1030
	LAP SPlice	300	440	580	730	880	1030
COMPRESSION $f_y = 600 \text{ MPa}$ $f_c = 40 \text{ MPa}$	LAP SPlice	200	290	380	480	580	680
	DEVELOPMENT LENGTH	200	280	360	450	540	630
	LAP SPlice	200	280	360	450	540	630
TENSION $f_y = 650 \text{ MPa}$ $f_c = 45 \text{ MPa}$	LAP SPlice	390	570	750	930	1110	1290
	DEVELOPMENT LENGTH	300	440	580	730	880	1030
	LAP SPlice	300	440	580	730	880	1030
COMPRESSION $f_y = 650 \text{ MPa}$ $f_c = 45 \text{ MPa}$	LAP SPlice	200	290	380	480	580	680
	DEVELOPMENT LENGTH	200	280	360	450	540	630
	LAP SPlice	200	280	360	450	540	630
TENSION $f_y = 700 \text{ MPa}$ $f_c = 50 \text{ MPa}$	LAP SPlice	390	570	750	930	1110	1290
	DEVELOPMENT LENGTH	300	440	580	730	880	1030
	LAP SPlice	300	440	580	730	880	1030
COMPRESSION $f_y = 700 \text{ MPa}$ $f_c = 50 \text{ MPa}$	LAP SPlice	200	290	380	480	580	680
	DEVELOPMENT LENGTH	200	280	360	450	540	630
	LAP SPlice	200	280	360	450	540	630
TENSION $f_y = 750 \text{ MPa}$ $f_c = 55 \text{ MPa}$	LAP SPlice	390	570	750	930	1110	1290
	DEVELOPMENT LENGTH	300	440	580	730	880	1030
	LAP SPlice	300	440	580	730	880	1030
COMPRESSION $f_y = 750 \text{ MPa}$ $f_c = 55 \text{ MPa}$	LAP SPlice	200	290	380	480	580	680
	DEVELOPMENT LENGTH	200	280	360	450	540	630
	LAP SPlice	200	280	360	450	540	630

207

TYPICAL BLOCK WALL FOOTING

UNHEATED AREA

Diagram illustrating the cross-section of a typical block wall footing for an unheated area. The footing is shown with dimensions and labels:

- BLOCK WALL**: SEE ARCH. DWG. (NON-LOAD BEARING.)
- SLAB ON GRADE OR ASPHALT**
- 2-15 CONT.**: 2-15 concrete layers, each 75 mm thick.
- 200 mm**: Width of the footing.
- 1200 mm**: Height of the footing.
- 75**: Thickness of each concrete layer.
- 200**: Width of the footing for 150 mm block wall.
- 300**: Width of the footing for 200 mm & 250 mm block wall.

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[illegible]

	ALEXANDRA PARK - BLOCK 11	
	TORONTO, ONTARIO	
project no:	13015	
scale:	AS NOTED	
drawn by:	H.W.	
reviewed by:	HOSMAN	
date started:	MARCH 2011	

GENERAL NOTES & TYPICAL DETAILS

S-001

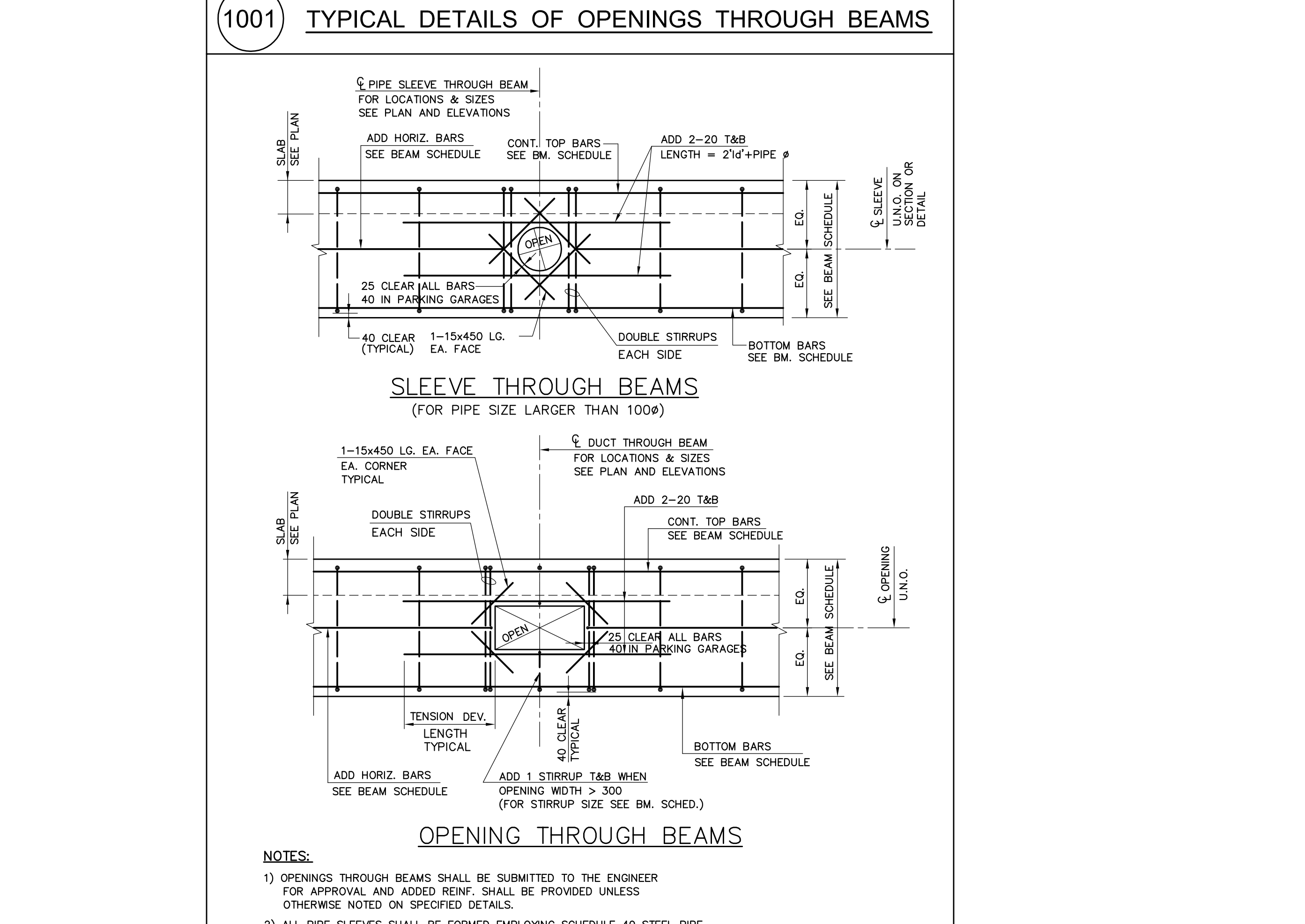
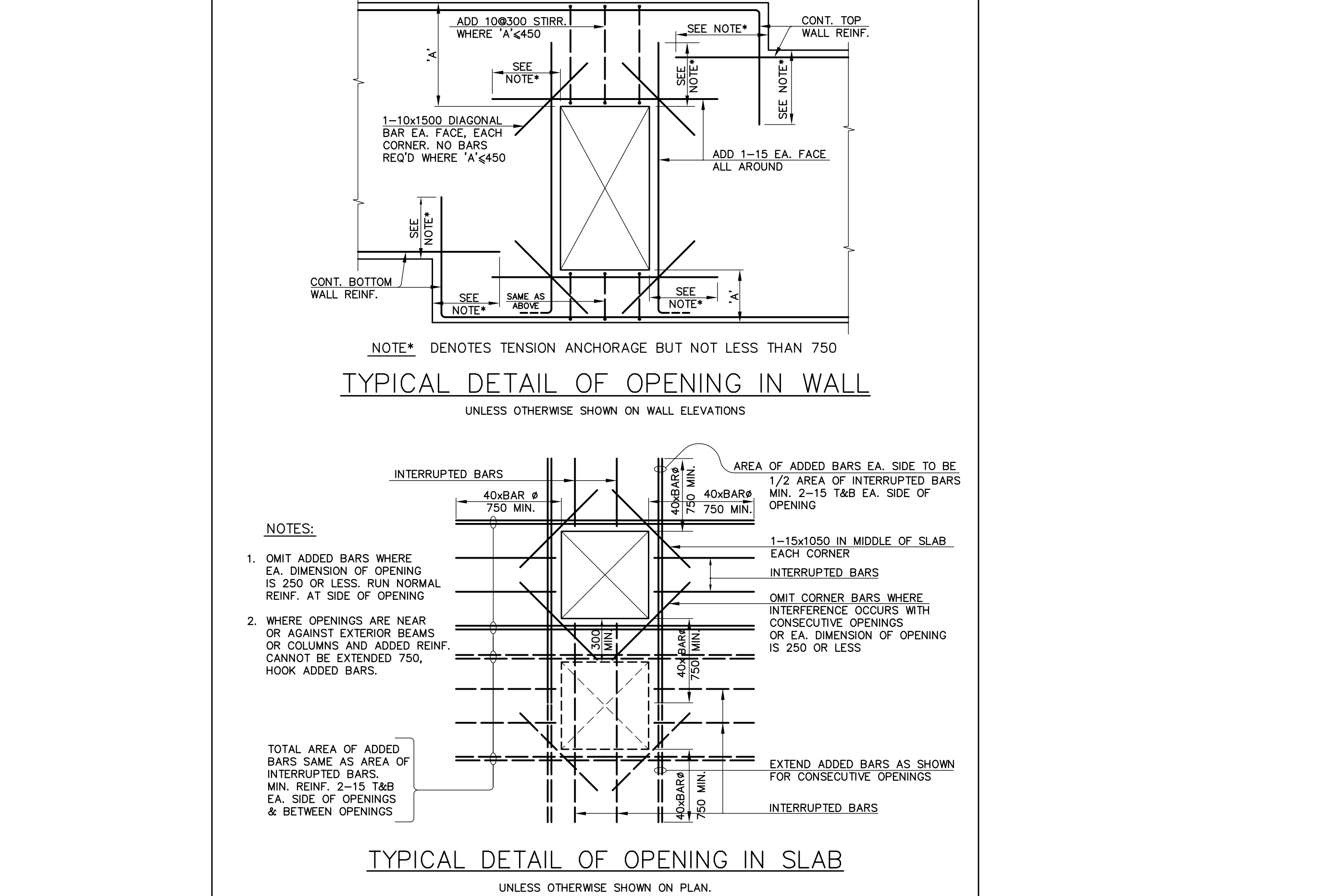
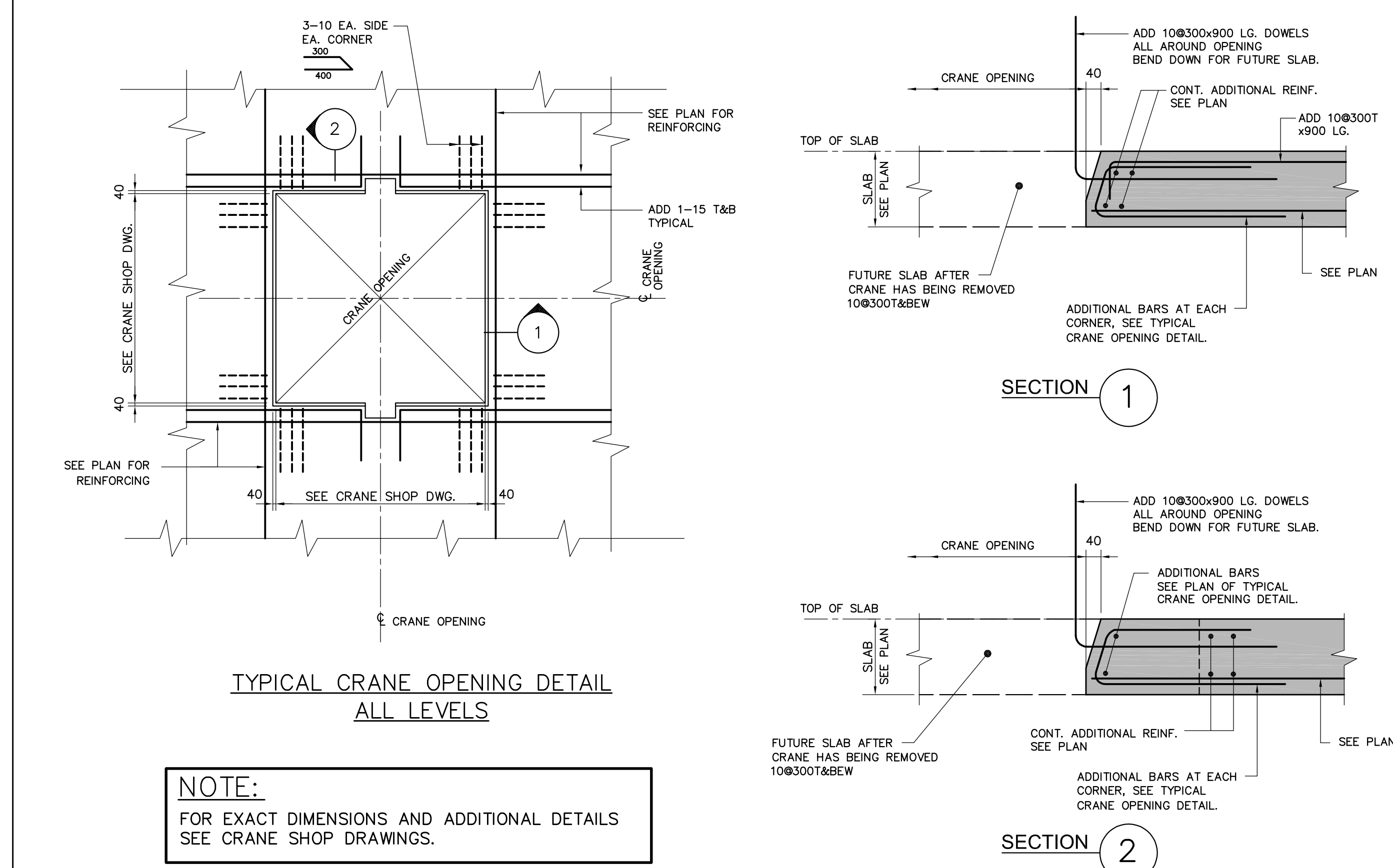
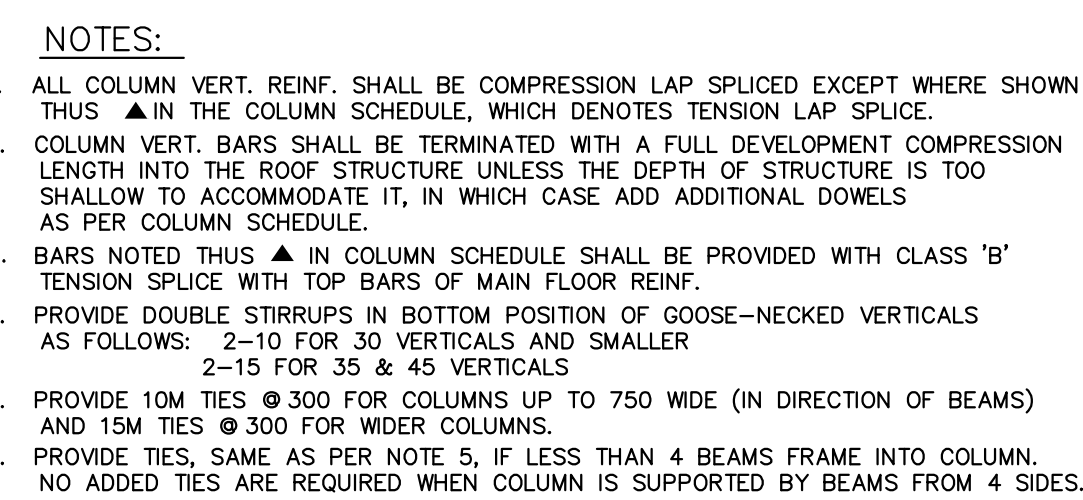
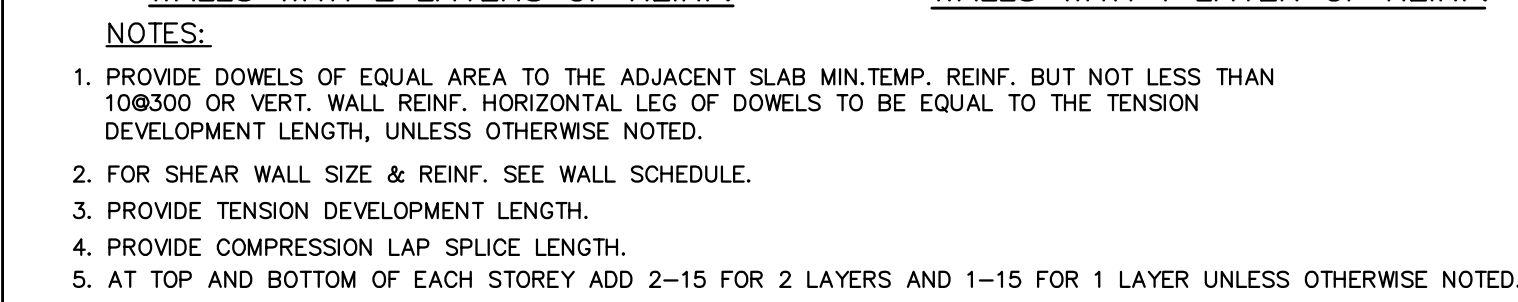


Figure 10 illustrates the reinforcement details for an exterior corner of a slab. The diagrams show the placement of reinforcement bars (30 x BAR ϕ 750 MIN.) and the required lap and development lengths (300) for the top and bottom reinforcement. The details are shown for the exterior corner top, exterior corner bottom, re-entrant corner top, and re-entrant corner bottom. A cross-section diagram shows the slab with added reinforcement (Ø300 MAX) and closed stirrups.

WALL 3 SIDES

450

450

100x300 DOWELS TOP & BOTTOM

50

150x300 U/N

100

150x300 U/N

2-20T8 CONT. TYP. U/NOTED

150x300 U/N

100

150x300 U/N

15 NOSING BAR IN EACH TREAD

0-25 (60) MIN

150x300 U/N

100

200

SEE PLAN

SEE PLAN

100

150x300 U/N

150x300 U/N

100

150x300 U/N

TEMP. REINF.

TEMP. REINF.

TEMP. REINF.

ADDED REINF. 2-20T8 CONT. TYP. U/NOTED

AT MID-LANDING

SEE WALL SCHEDULE

LEDGE FOR P.C. STAIR
SEE SHOP DWGS.

10B150

ADD 2-20 TAB
CONT. U/N ON
PLAN

SEE PLAN

2-15
CONT.

SEE P.C. STAIR
SHOP DWGS.

1000

1000

600

200

100
MIN.

1000

50mm MIN. CHASE
TYP. ALL AROUND
OR POCKETS AS
NOTED ON PLAN

MID-LANDING SLAB

LEDGE FOR P.C. STAIR
SEE SHOP DWGS.

10B150

ADD 2-20 TAB
CONT. U/N ON
PLAN

SEE PLAN

2-15
CONT.

SEE P.C. STAIR
SHOP DWGS.

100

600

200

100
MIN.

EDGE OF SLAB: UP FLIGHT

LEDGE FOR P.C. STAIR
SEE SHOP DWGS.

10B150

ADD 2-20 TAB
CONT. U/N ON
PLAN

SEE PLAN

2-15 CONT.

SEE P.C. STAIR
SHOP DWGS.

100

600

200

100
MIN.

PRECAST STAIR

EDGE OF SLAB: DOWN FLIGHT

ADD 1-15 CONT.
EA. SIDE
SEE P.C. STAIR
SHOP DWGS.

PRECAST STAIR

1500

SEE PLAN

450

200

ADD 2-20
TAB CONT.

450

BEARING ON STRUCTURAL SLAB

PRECAST STAIR

1-15000T
EA. SIDE

SEE P.C. STAIR
SHOP DWGS.

SEE PLAN

1500000T

WELDED WIRE FABRIC
WHERE SPECIFIED

300

11

2

300

2-11
CONT.

450

200

450

BEARING ON SLAB ON GRADE

IN FRAMED SLABS

ADD. 2-15 MIN. OR 1500x1500 L.G. ABOVE & BELOW KEY

40

ADD. 10-60 MIN. AT 200 L.G. OR 200x800x1500 L.G. WHERE THERE ARE NO CONT. TOP BARS

ALL BARS TO RUN THROUGH

IN BEAMS

DISTANCE BETWEEN CONSTRUCTION JOINTS = 18000 MM. UNLESS OTHERWISE APPROVED BY THE ENGINEER

TENSION LAP SPUCE

40

P.V.C. WATER BAR IN SUB-GRADE EXTERIOR WALLS OR EXTERIOR P.V.C. WATER BAR AT EXTERIOR FACE

ADD. 15x1300 L.G. DOWELS EA. FACE EQUALLY SPACED BETWEEN MAIN HOR. WALL REINF.

ROUGHEN SURFACE OF CONC. AND COAT WITH CEMENT SLURRY BEFORE SECOND CASTING

IN WALLS

- CONSTRUCTION JOINT SHALL BE PLACED 1200 MM. AWAY FROM OPENINGS OR FACE OF SUPPORTING PIECE
- WALLS SUPPORTED ON PIERS OR CAISSONS SHALL HAVE CONSTRUCTION JOINT LOCATED AT MIDSPAN, UNLESS OTHERWISE APPROVED BY THE ENGINEER
- PROVIDE TENSION LAP SPUCE AS INDICATED AND FOR ALL CONTINUOUS TOP & BOTTOM BARS THROUGH CONSTRUCTION JOINT.

A. TYPICAL DETAIL OF TOP STEP IN BEAM

δd = TENSION DEVELOPMENT LENGTH

B. TYPICAL DETAIL OF BOTTOM STEP IN BEAM

C. TYPICAL DETAIL OF CRANKED BEAM

[illegible]

Diagram illustrating a typical expansion joint in walls. The joint is shown in cross-section, featuring a central vertical gap. The gap is filled with a compressible filler material. On either side of the filler, there is a 150mm wide PVC water stop. The joint is sealed with sealant on the exterior face (EA. FACE). The joint is located at a control joint, where the horizontal reinforcing is cut 50% at each control joint location. The joint is also filled with a flexible sealant. The joint is provided with a reglet 100mm x 20mm deep for the full height of the wall. The joint is located on the inside face of the wall.

TYPICAL EXPANSION JOINT IN WALLS

Labels in diagram:

- SEALANT EA. FACE
- 150 WIDE PVC WATER STOP
- COMPRESSIBLE FILLER
- CUT 50% OF HORIZONTAL INSIDE FACE REINFORCING AT EACH CONTROL JOINT LOCATION
- FILL REGLET WITH FLEXIBLE SEALANT
- PROVIDE REGLET 100mm x 20mm DEEP FOR FULL HEIGHT OF WALL
- INSIDE FACE

TYPICAL CONTROL JOINT DETAIL

IN FOUNDATION WALLS - PLAN DETAIL

Labels in diagram:

- CONTROL JOINTS AT 4500 C/MX. (CONTRACTORS TO SUBMIT CONTROL JOINT LOCATIONS FOR APPROVAL)

FOR DETAILS OF COVER
PLATE SEE ARCH. DWGS.

EXPANSION JOINT
WITH COMPRESSIBLE
MATERIAL. (SEE
PLAN FOR
WIDTH)

40
COVER

ADD 2-1/2 " (W4000 WELDED
TO TOP OF BEARING PLATE)

SHEAR FRICTION REINF.
(SEE NOTE #3)
CLOSED TIES $\square$

TOP BM. REINFORCING
(SEE BM. SCHED.)

25 CHAMFER
40 COVER

MAIN TENSION REINF.
(W4000 WELDABLE STEEL
SEE NOTE #3)

MIN. STRAIGHT TENSION
DEV'T. LENGTH (TOP BARS)

FOR STIRRUP SIZE & SPACING
SEE BEAM SCHEDULE

BOTTOM BM. REINF.
SEE BM. SCHED.

HANGER REINF.—CLOSED STIRRUPS
SEE NOTE #3

CORBEL

SEE SECTION
OF BEARING

COLUMN
SCHEDULE

SEE BEAM SCHEDULE

NOTES

1. PROVIDE GREATER OF THE FOLLOWING:
 - A. EXTEND TO CRACK LINE 0.2 L
 - B. MIN. TENSION EMBEDMENT WITH STD. END HOOK C. 300mm
2. DISTRIBUTE SHEAR FRICTION REINFORCEMENT EQUALLY WITHIN HEIGHT OF 2/3 h.
3. FOR SIZE AND NUMBERS OF SHEAR FRICTION, MAIN TENSION AND HANGER REINFORCEMENT, SEE RELATED SECTIONS.

SEE ARCH. DWG. FOR JOINT FILLER

PRECAST STAIR

FIN. FLOOR

ADD 2-15 CONT.

ADD 2-15 CONT.

SEE WALL SCHED. FOR REINF.

ADD 2-20 TAB CONT. TENSION ANCHORED INTO FLOOR SLAB & WALL AT EACH END OF WALL.

PRECAST STAIR

FLOOR SLAB

IN SLABS

Diagram showing the cross-section of a slab with reinforcement details. Key dimensions and labels include:

- DEPTH OF SLAB: D
- 50mm TYP. (top and bottom bars)
- SEE NOTE #3
- SEE NOTE #2
- 22mm TYP. (bottom bars)
- WIDTH OF CLOSURE STRIP (SEE NOTE #1)
- 20mm TYP. (bottom bars)
- BOTTOM BARS AS SPECIFIED (SEE PLAN)
- CONC. TOP BARS (SEE BM. SCHEDULE) OR PROVIDE 2-20 MIN. CONT. OR 20Ø300
- STRIPBARS OF BEAM CLOSURE STRIP FOR SIZE & SPACING SEE BEAM SCHEDULE
- 50mm TYP. (top bars)
- SEE NOTE #2
- 50mm MAX. (width of closure strip)
- 20mm TYP. (bottom bars)
- BEAM REINFORCEMENT (SEE BM. SCHED.)
- ADD 2-20 MIN. OR 20Ø300 ABOVE & BELOW KEY
- S = SPACING AS IN BM. SCHED.
- OR LESS
- 100mm MAX. (width of closure strip)

IN BEAMS

Diagram showing the cross-section of a beam with reinforcement details. Key dimensions and labels include:

- DEPTH OF BEAM: D
- 50mm TYP. (top bars)
- SEE NOTE #2
- 50mm MAX. (width of closure strip)
- 20mm TYP. (bottom bars)
- BEAM REINFORCEMENT (SEE BM. SCHED.)
- ADD 2-20 MIN. OR 20Ø300 ABOVE & BELOW KEY
- S = SPACING AS IN BM. SCHED.
- OR LESS
- 100mm MAX. (width of closure strip)

CLOSURE STRIP NOTES:

1. WIDTH OF CLOSURE STRIP SHALL BE EQUAL TO THE LENGTH OF BASIC TENSION LAP SPlice CLASSE (DIA.) BAR CROSSING THE CLOSURE STRIP PLUS 50mm.
2. PROVIDE COMPRESSION LAP SPlice WHERE INDICATED.
3. ADDITIONAL TOP BARS OF SLAB CLOSURE STRIP BE AS FOLLOWS:

SLAB THICKNESS	ADDITIONAL TOP BARS
UP TO 230mm	10mØ240mm
230mm TO 300mm	15mØ360mm
THICKER THAN 300mm	AREA OF STEEL SHALL BE EQUAL TO 0.0020 X CONCRETE CROSS SECTION AREA PER METRE WIDTH.
4. CLOSURE STRIP SHALL BE CAST 21 DAYS (MIN.) AFTER THE LATER ADJACENT SLAB IS CAST, UNLESS SPECIFIED OTHERWISE ON THE DRAWING.
5. AFTER CASTING, THE CLOSURE STRIP FORMWORK SHALL NOT BE REMOVED FOR A PERIOD OF 7 DAYS (MIN.) RESHORE IMMEDIATELY AFTER REMOVAL FOR A FURTHER 7 DAYS (MIN.) COLD WEATHER REQUIREMENTS FOR CONCRETE PROTECTION SHALL BE PROVIDED AS REQUIRED.

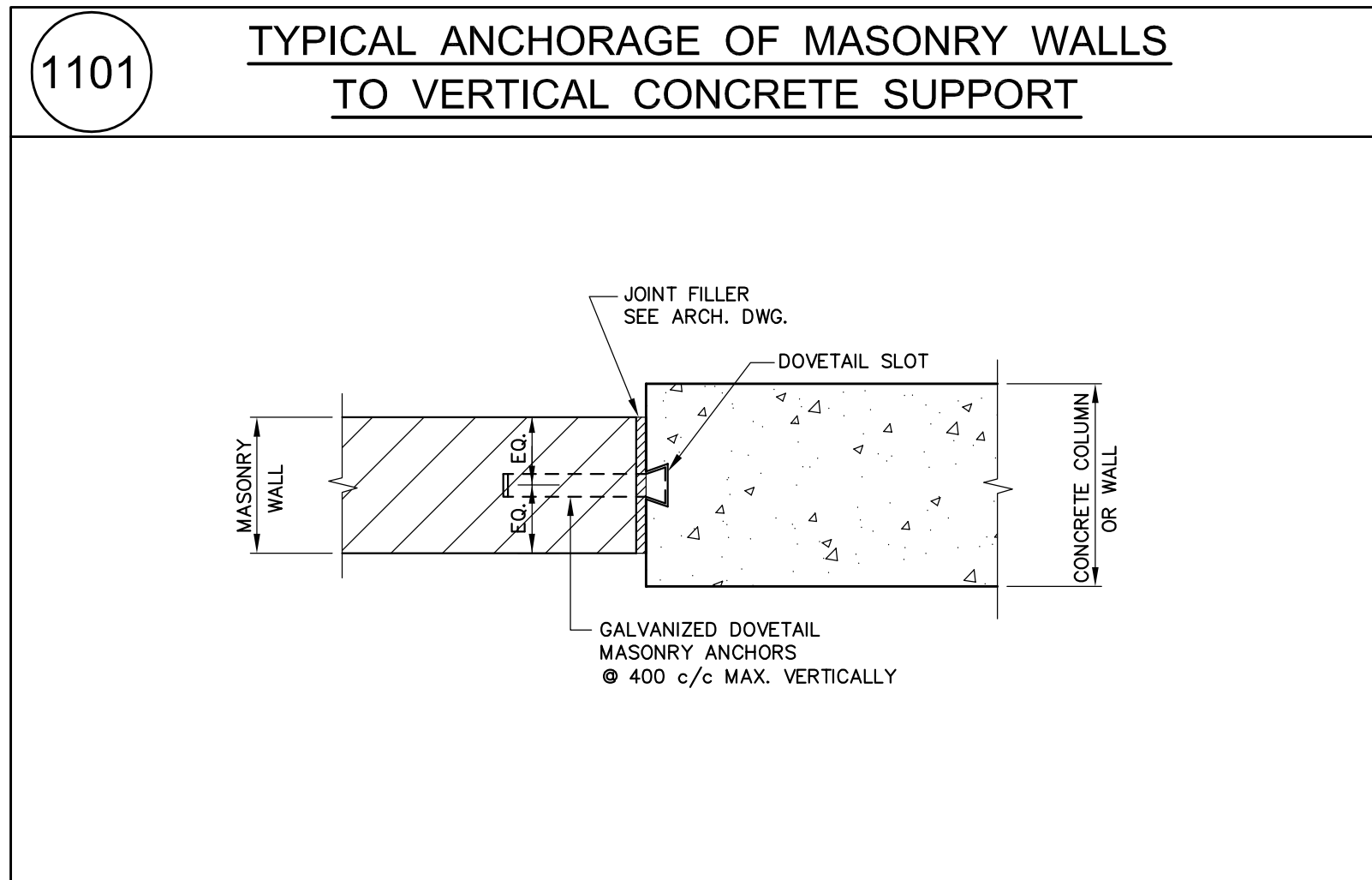
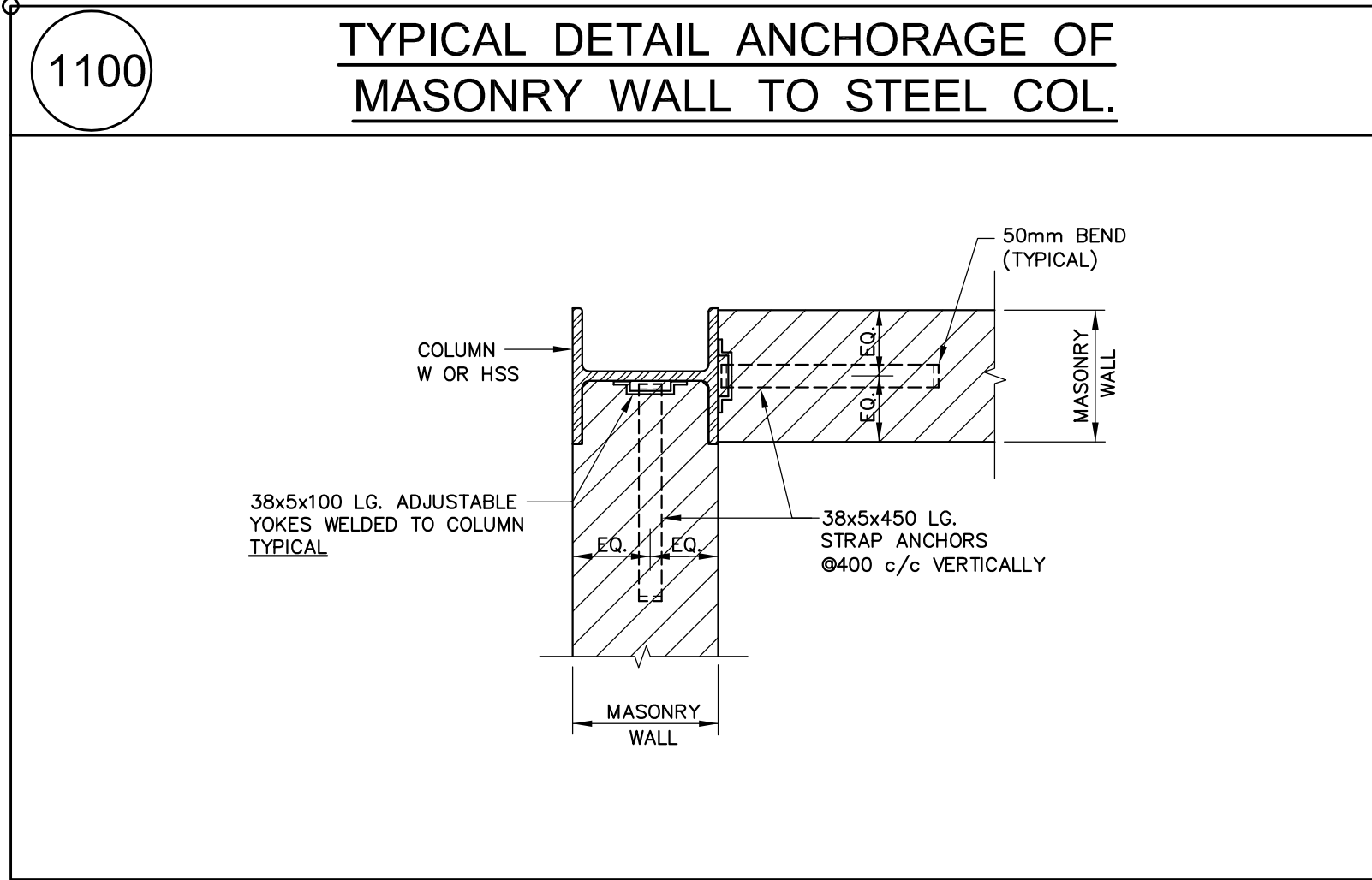
[illegible]

Figure 10: Typical Details of Concrete Joints. The figure includes a plan view at the top and five cross-sectional details below, labeled from left to right: EXPANSION JOINT, CONTROL JOINT, CONSTRUCTION JOINT, CONTROL JOINT, and EXPANSION JOINT.

The plan view shows a wall section with a total length of 9000 MAX. The joint is centered, with 4500 MAX. on each side. A splice with 1/2 area of count top bars (2-15x1000 L.G.) is indicated at the center. The joint is labeled "STOP ALTERNATE BARS EACH FACE" and "ALL BARS 50mm CLR.".

The cross-sectional details show the following dimensions and materials:

- EXPANSION JOINT (Left):** Shows a 40mm gap between two concrete sections. The gap is filled with "COMPRESSIVE FILLER" and sealed with "SEALANT". The concrete sections are 20mm thick.
- CONTROL JOINT (Second from Left):** Shows a 40mm gap between two concrete sections. The gap is filled with "NEOPRENE BONDED TO FORMS". The concrete sections are 25mm thick.
- CONSTRUCTION JOINT (Third from Left):** Shows a 40mm gap between two concrete sections. The gap is filled with "NEOPRENE BONDED TO FORMS". The concrete sections are 25mm thick.
- CONTROL JOINT (Fourth from Left):** Shows a 40mm gap between two concrete sections. The gap is filled with "NEOPRENE BONDED TO FORMS". The concrete sections are 25mm thick.
- EXPANSION JOINT (Right):** Shows a 40mm gap between two concrete sections. The gap is filled with "COMPRESSIVE FILLER" and sealed with "SEALANT". The concrete sections are 20mm thick.



1102 LINTEL SCHEDULE

MAXIMUM CLEAR SPAN	BLOCK LINTEL	MAXIMUM CLEAR SPAN	STEEL LINTEL FOR EXTERIOR WALL
2000	FOR INTERIOR WALL 1-15 CONT. 90 OR 140	1800	BRICK L-102x102x8
2400	FOR INTERIOR WALL 2-15 CONT. 90 OR 140	2000	BRICK L-102x102x8 + L-51x51x4.8 20 MAX.
2400	FOR EXTERIOR & CAVITY WALL 2-15 CONT. 90 OR 140	2400	BRICK L-152x102x8 (LLV) + L-51x51x4.8 20 MAX.
3000	FOR EXTERIOR & CAVITY WALL 4-15 CONT. 10#400 STIRRUPS 2-20 CONT. 90 OR 140	3000	BRICK L-152x102x10 (LLV) + L-51x51x4.8 20 MAX.
3600	FOR EXTERIOR & CAVITY WALL 4-15 CONT. 10#400 STIRRUPS 2-25 CONT. 90 OR 140	3600	BRICK L-203x102x13 (LLV) + L-51x51x4.8 20 MAX.

NOTES:

- MINIMUM END BEARING FOR LINTELS SHALL BE 200mm.
- CONCRETE FILL SHALL BE $f_c = 25\text{MPa}$.
- PROVIDE TEMPORARY SHORING TO SUPPORT MASONRY OVER OPENINGS UNTIL CONCRETE HAS DEVELOPED FULL STRENGTH.

NOTES:

- MINIMUM END BEARING FOR LINTELS SHALL BE 150mm.
- ALL ANGLES SHALL BE HOT-DIPPED GALVANIZED.
- STRUCTURAL STEEL SHALL CONFORM TO CAN/CSA G40.21-04, 300W.

1103 TYPICAL STEEL LINTELS IN NON-LOAD BEARING MASONRY WALLS

MAX. CLEAR SPAN	200	250	300	315	365
1200	2 $\angle$ - 90x90x8	1 $\angle$ - 90x90x8 2 $\angle$ - 125x90x8 (LLH)	3 $\angle$ - 90x90x8	1 $\angle$ - 100x100x8 2 $\angle$ - 100x100x8 3 $\angle$ - 90x90x8	1 $\angle$ - 100x100x8 2 $\angle$ - 150x100x8 (LLH) 3 $\angle$ - 90x90x8
1800	2 $\angle$ - 100x90x8 (LLV)	1 $\angle$ - 100x90x8 (LLV) 2 $\angle$ - 125x125x8	3 $\angle$ - 100x90x8 (LLV)	1 $\angle$ - 100x100x8 2 $\angle$ - 100x100x8 (LLV) 3 $\angle$ - 100x90x8 (LLV)	1 $\angle$ - 100x100x8 2 $\angle$ - 150x100x8 (LLH) 3 $\angle$ - 100x100x8
2400	2 $\angle$ - 125x90x8 (LLV)	1 $\angle$ - 125x90x8 (LLV) 2 $\angle$ - 125x125x10 3 $\angle$ - 125x90x8 (LLV)	3 $\angle$ - 125x90x8 (LLV)	1 $\angle$ - 125x90x8 (LLV) 2 $\angle$ - 125x90x8 (LLV) 3 $\angle$ - 125x90x8 (LLV)	1 $\angle$ - 125x90x8 (LLV) 2 $\angle$ - 150x100x8 (LLH) 3 $\angle$ - 125x90x8 (LLV)
3000	2 $\angle$ - 125x100x10 (LLV)	1 $\angle$ - 150x100x10 (LLV) 2 $\angle$ - 150x150x10	3 $\angle$ - 150x100x10 (LLV)	1 $\angle$ - 150x100x10 (LLV) 2 $\angle$ - 150x100x10 (LLV) 3 $\angle$ - 150x100x10 (LLV)	1 $\angle$ - 150x100x10 (LLV) 2 $\angle$ - 150x100x10 (LLH) 3 $\angle$ - 150x100x10 (LLV)

NOTES:

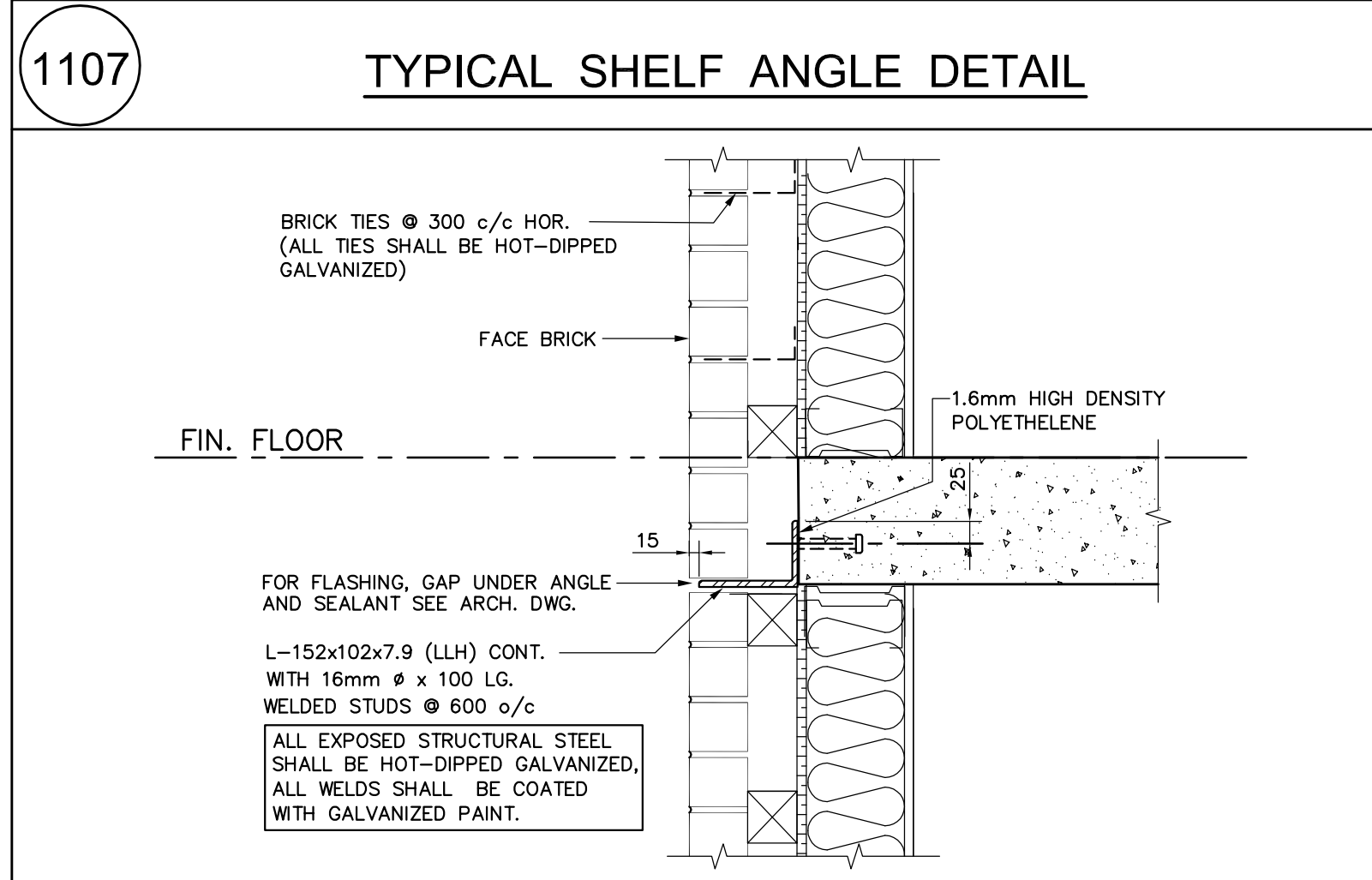
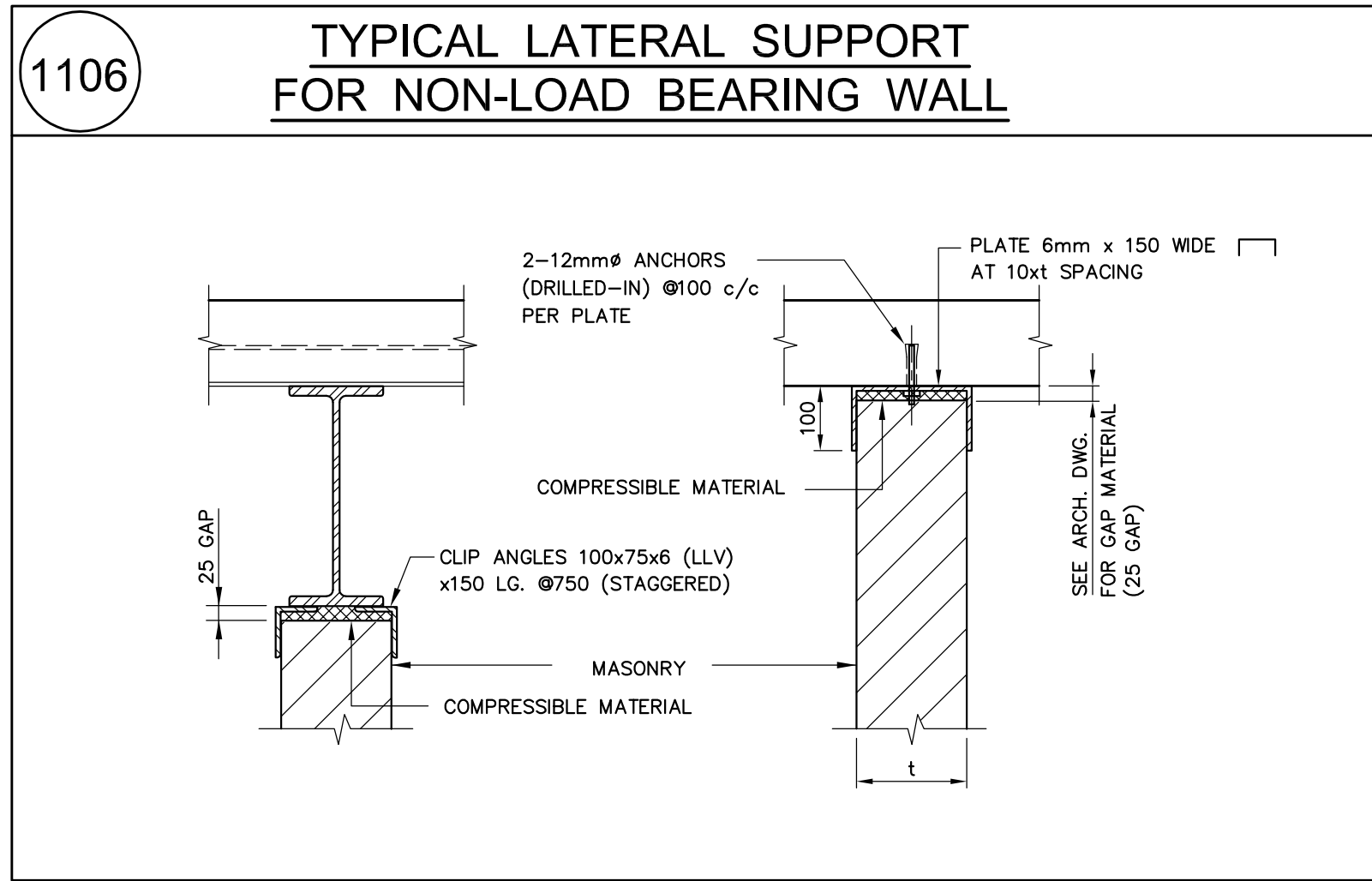
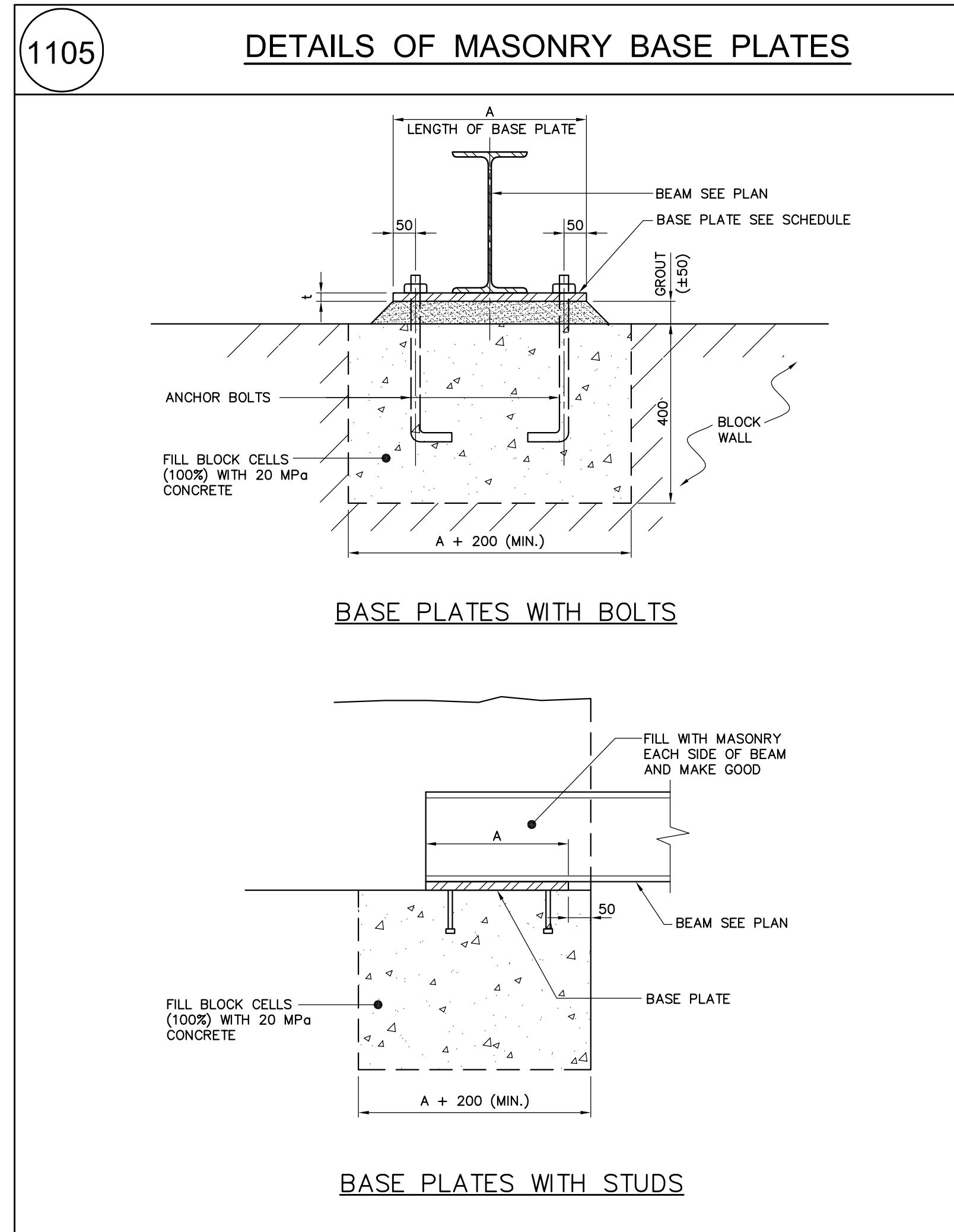
- FOR 150 WALL USE $\angle$ OF 250 WALL ABOVE.
- MIN. END BEARING FOR LINTELS SHALL BE 150mm.
- BACK TO BACK ANGLES SHALL BE BOLTED OR WELDED TOGETHER WHEN CLEAR SPAN EXCEEDS 1800mm.
- ALL ANGLES SHALL BE HOT-DIPPED GALVANIZED IF EXPOSED TO WEATHER.
- PROVIDE L-90x90x10 WELDED TO STEEL COLUMN OR BOLTED TO CONCRETE COLUMN OR WALL TO SUPPORT LINTEL WHERE OPENING ABUTS COLUMN OR WALL.
- STRUCTURAL STEEL SHALL CONFORM TO CAN/CSA G40.21-04, 300W.

1104 STEEL LINTELS FOR NON-LOAD BEARING WALLS OF HOLLOW CONCRETE BLOCK (ANY AGGREGATE)

STEEL ANGLES CLEAR SPAN MAX.	90	140	190	240	280
1200	2 $\angle$ - 51x38x4.8LLV.	2 $\angle$ - 64x64x6.4	2 $\angle$ - 89x76x6.4LLV.	1 $\angle$ - 102x76x6.4LLH. 1 $\angle$ - 127x76x6.4LLH.	3 $\angle$ - 89x76x6.4LLH.
1800	2 $\angle$ - 51x38x4.8(LLV.)	2 $\angle$ - 64x64x6.4	2 $\angle$ - 89x76x6.4LLV.	1 $\angle$ - 102x76x6.4LLH. 1 $\angle$ - 127x76x6.4LLH.	3 $\angle$ - 89x76x6.4LLH.
2000		2 $\angle$ - 89x64x6.4LLV.	2 $\angle$ - 89x89x6.4	1 $\angle$ - 102x89x7.9LLH. 1 $\angle$ - 127x76x7.9LLH.	3 $\angle$ - 89x76x6.4LLH.
2400		2 $\angle$ - 89x64x9.5LLV.	2 $\angle$ - 127x89x6.4LLV.	1 $\angle$ - 152x102x7.9LLV. 1 $\angle$ - 127x127x7.9	3 $\angle$ - 127x89x6.4LLV.

NOTES:

- STRUCTURAL STEEL MATERIALS SHALL BE CAN/CSA G40.21-04 300W.
- MINIMUM BEARING LENGTH FOR LINTELS SHALL BE 150mm.
- CONNECT ANGLES AT 600 mm c/c BY WELDING OR BOLTING FOR ANGLES WITH A TOTAL LENGTH OF 1800mm OR MORE.
- PROVIDE L-90x90x10 WELDED TO STEEL COLUMN OR BOLTED TO CONCRETE COLUMN OR WALL TO SUPPORT LINTEL WHERE OPENING ABUTS COLUMN OR WALL.
- ALL ANGLES SHALL BE HOT-DIPPED GALVANIZED IF EXPOSED TO WEATHER.



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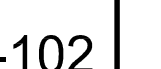
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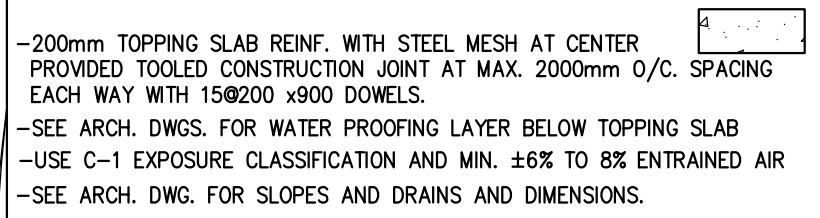
NO.	Issued / Revised	Date
1	ISSUED FOR PERMIT	2014-05-28
2	RE ISSUED FOR PERMIT	2014-07-10
3	ISSUED FOR FORMING TRENCH	2014-06-27
4	CONSTRUCTION REVIEW	2014-03-11
5	RE ISSUED FOR PERMIT	2014-07-03

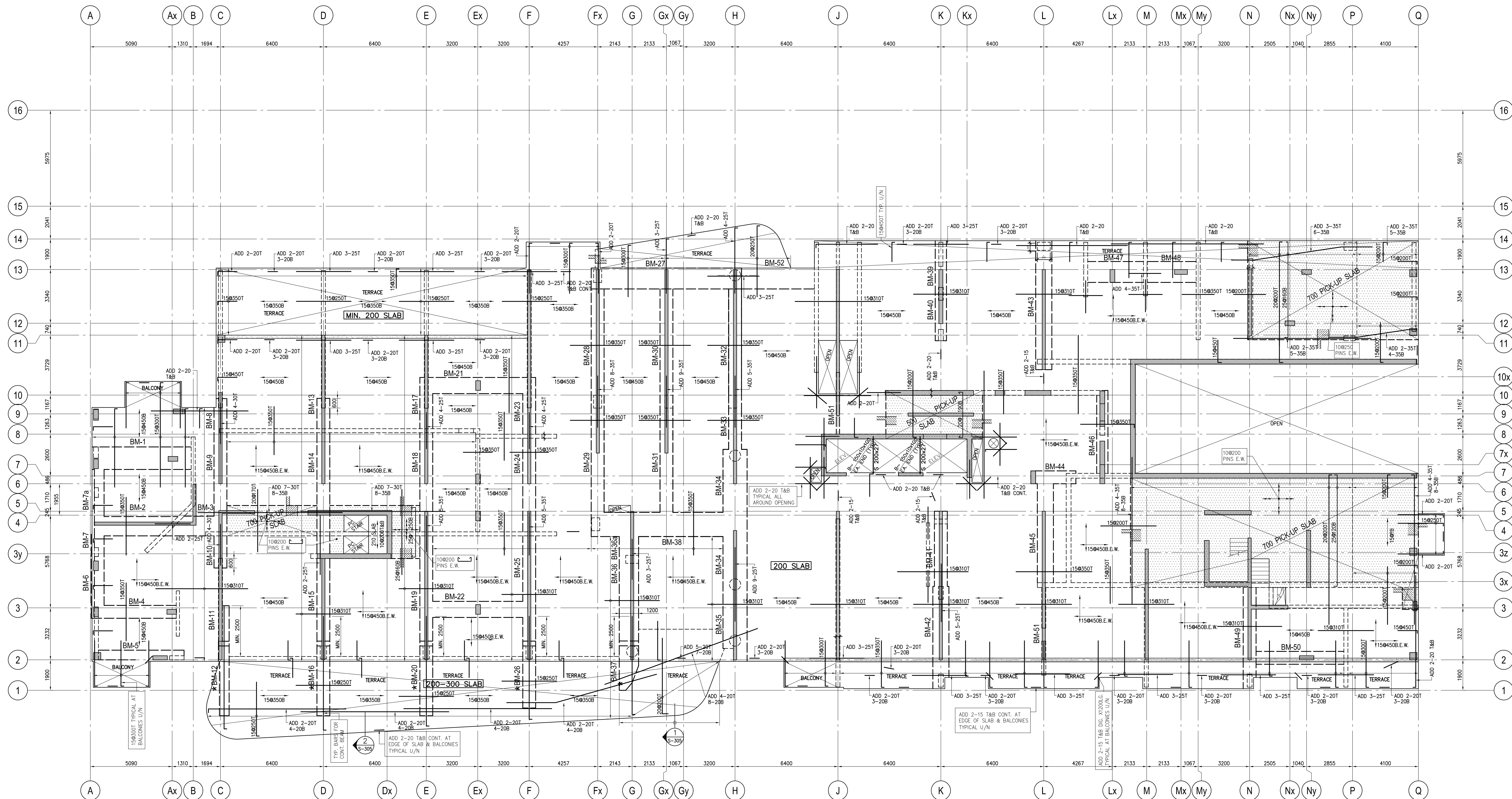
ALEXANDRA PARK - BLOCK 11
TORONTO, ONTARIO

project no:
scale:
drawn by:
reviewed by:
date started:

TYPICAL DETAILS







2ND FLOOR FRAMING PLAN

SCALE 1 : 100

- TOP OF SLAB IS AT ELEVATION AS SHOWN ON ARCH. DRAWINGS EXCEPT AS CROSSED AND NOTED ON PLAN.
- CONCRETE STRENGTH AT 28 DAYS SHALL BE:
FOR WALLS AND COLUMNS SEE SCHEDULE
FOR EXT. SLABS 35 MPa
FOR INTERIOR SLABS 25 MPa
FOR BEAMS 35 MPa
FOR PICK-UP SLABS 35 MPa
CONCRETE EXPOSED TO ELEMENTS SHALL BE 35 MPa WITH 6% TO 8% ENTRAINED AIR.
- FLOOR SLABS ARE DESIGNED FOR FOLLOWING LOADING CONDITIONS :

	S.D.	LL
STAIRS & BALCONIES	0.50 KPa	4.80 KPa
LOOKERS & STORAGE *	1.30 KPa	4.80 KPa
RESIDENTIAL	1.3 KPa	1.9 KPa
TOILETS	1.30 KPa	2.40 KPa
TERRACES	5.0 KPa	4.80 KPa

- MINIMUM YIELD STRESS FOR REINFORCING STEEL SHALL BE 400 MPa.
- TEMPERATURE REINFORCING FOR :
200 SLAB IS 100250
500 SLAB IS 150200
700 SLAB IS 200210.
- NO OPENINGS LARGER THAN 300mm x 300mm ARE ALLOWED IN SLAB OTHER THAN THOSE SHOWN ON DRAWINGS.
- SEE TYPICAL DETAILS ON DRAWINGS S-001 TO S-006.
- SEE GENERAL NOTES ON DRAWING S-001.
- REFER TO ARCH. DRAWINGS FOR SLOPES OF SLAB.
- FOR COLUMN & WALL SCHEDULE SEE DRAWINGS S-301 TO S-306.
- COORDINATE BEAM DEPTH AT DOOR OPENINGS WITH ARCH. DRAWINGS.
- EXTEND TEMP. REINF. TO END OF BALCONIES/OVERHANGS.
- TOP BARS TERMINATING AT EDGE OF SLAB TO HAVE 180° HOOK.

2ND FLOOR BEAM SCHEDULE (fc'=35MPa)

MARK	WIDTH	DEPTH	REINFORCEMENT		SIZE	TYPE	SPACING EACH END	REMARKS
			BOTTOM CONT.	TOP CONT.				
BM-1	1400	700	12-25	6-20	10	10110, 60230, 0400		ADD 1-1SHEF
BM-2	1200	700	8-20	5-20	10	10185, 0370		ADD 1-1SHEF
BM-3	1200	700	8-20	5-20	10	10185, 0370		ADD 1-1SHEF
BM-4	800	700	5-25	4-20	15	10770, 60140, 0400		ADD 1-1SHEF
BM-5	600	700	5-30	4-20	15	10150, 0300		ADD 1-1SHEF
BM-6	600	700	4-20	4-20	10	10110, 0220		ADD 1-1SHEF
BM-7	600	700	4-25	4-20	10	10110, 0220		ADD 1-1SHEF
BM-7a	600	700	4-20	4-20	10	10125, 0250		ADD 1-1SHEF
BM-8	800	700	5-20	7-30	10	10200, 0400		ADD 1-1SHEF
BM-9	800	700	7-35	7-30	15	10950, 21095, 0235		ADD 1-1SHEF
BM-10	800	700	5-20	7-30	10	10200, 0400		ADD 1-1SHEF
BM-11	600	700	6-25	6-30	10	10200, 0400		ADD 1-1SHEF
BM-12	600	700	6-25	6-30	15	10175, 0350		ADD 1-1SHEF *
BM-13	800	700	6-25	6-25	10	10150, 0300		ADD 1-1SHEF
BM-14	800	700	6-25	6-25	15	10135, 50270, 0350		ADD 1-1SHEF
BM-15	800	700	6-30	6-25	15	10135, 50270, 0350		ADD 1-1SHEF
BM-16	800	700	5-20	6-30	15	10175, 0350		ADD 1-1SHEF *
BM-17	800	700	5-20	5-20	10	10150, 0330		ADD 1-1SHEF
BM-18	800	700	6-25	6-25	15	10110, 60220, 0350		ADD 1-1SHEF
BM-19	800	700	7-25	6-20	15	1085, 80175, 0270		ADD 1-1SHEF
BM-20	800	700	5-20	6-30	15	10175, 0350		ADD 1-1SHEF *
BM-21	1000	700	16-35 2 LAYERS	6-20	15	10150, 0300		ADD 1-1SHEF
BM-22	1000	700	16-35 2 LAYERS	6-20	15	10150, 0300		ADD 1-1SHEF
BM-23	800	700	5-20	5-20	10	10150, 0330		ADD 1-1SHEF
BM-24	800	700	6-25	5-20	15	10110, 60220, 0350		ADD 1-1SHEF
BM-25	800	700	7-25	5-20	15	1085, 80175, 0270		ADD 1-1SHEF
BM-26	800	700	5-20	6-30	15	10175, 0350		ADD 1-1SHEF *

* CAMBER UP END OF THE BEAM 20 mm

2ND FLOOR BEAM SCHEDULE (fc'=35MPa)

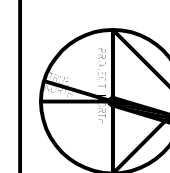
MARK	WIDTH	DEPTH	REINFORCEMENT		SIZE	TYPE	SPACING EACH END	REMARKS
			BOTTOM CONT.	TOP CONT.				
BM-27	1300	1000	10-35 3 LAYERS	8-30	15	10100, 60200, 0250		ADD 1-1SHEF
BM-28	800	700	8-35	6-20	15	1085, 80175, 0275		ADD 1-1SHEF
BM-29	800	700	8-25	6-20	15	10100, 100200, 0350		ADD 1-1SHEF
BM-30	800	700	8-35	6-20	15	1085, 80175, 0275		ADD 1-1SHEF
BM-31	800	700	8-25	6-20	15	10100, 100200, 0350		ADD 1-1SHEF
BM-32	800	700	10-30	6-20	15	10770, 100140, 0200		ADD 1-1SHEF
BM-33	800	700	10-20	6-20	15	10175, 0350		ADD 1-1SHEF
BM-34	1500	700	10-35	8-20	15	1065, 180130, 0300		ADD 1-1SHEF
BM-35	1500	700	14-20	6-20	15	10175, 0350		ADD 1-1SHEF
BM-36	1200	700	9-25	6-20	15	10175, 0350		ADD 1-1SHEF
BM-36a	1200	700	9-25	6-20	15	10145, 50290, 0350		ADD 1-1SHEF
BM-37	1200	700	9-25	9-25	15	10175, 0350		ADD 1-1SHEF *
BM-38	1500	700	13-30	10-20	15	10150, 0300		ADD 1-1SHEF
BM-39	700	700	4-25	4-20	10	10135, 0270		ADD 1-1SHEF
BM-40	700	700	4-25	4-20	10	10135, 0270		ADD 1-1SHEF
BM-41	800	700	6-20	6-20	15	10150, 0300		ADD 1-1SHEF
BM-42	1000	700	9-35	6-20	15	1080, 80165, 0250		ADD 1-1SHEF
BM-43	1000	800	27-35 3 LAYERS	7-25	15	10770, 180140, 0300		ADD 1-1SHEF
BM-44	800	700	7-25	4-25	15	10100, 0200		ADD 1-1SHEF
BM-45	1000	700	16-35 2 LAYERS	6-25	15	10100, 100200, 0250		ADD 1-1SHEF
BM-46	700	700	4-25	4-20	10	10150, 0300		ADD 1-1SHEF
BM-47	1300	800	13-35	7-30	15	1085, 0170		ADD 1-1SHEF
BM-48	1300	800	13-35	7-30	15	10125, 0250		ADD 1-1SHEF
BM-49	800	800	14-35 2 LAYERS	7-20	15	1080, 100165, 0250		ADD 1-1SHEF
BM-50	800	700	12-35 2 LAYERS	7-20	15	1085, 80170, 0195		ADD 1-1SHEF
BM-51	200	1000	4-25	2-20	10	10150, 0300		ADD 150300 HEF
BM-52	1300	500	8-25	5-20	10	10150, 0300		ADD 1-1SHEF

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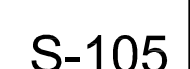
FIRST FLOOR ELEV. 93.60m

NO.	Issued / Revised	Date
1	ISSUED FOR PERMIT	2014/05/28
2	RE ISSUED FOR PERMIT	2014/05/16
3	ISSUED FOR FORMING TRENCH	2014/06/07
4	CONSTRUCTION REVIEW	2014/07/01
5	RE ISSUED FOR PERMIT	2014/07/01



project no: 13015
scale: 1:100
drawn by: H.W.
checked by: H.W.
date started: MARCH 2014

2ND FLOOR
FRAMING PLAN

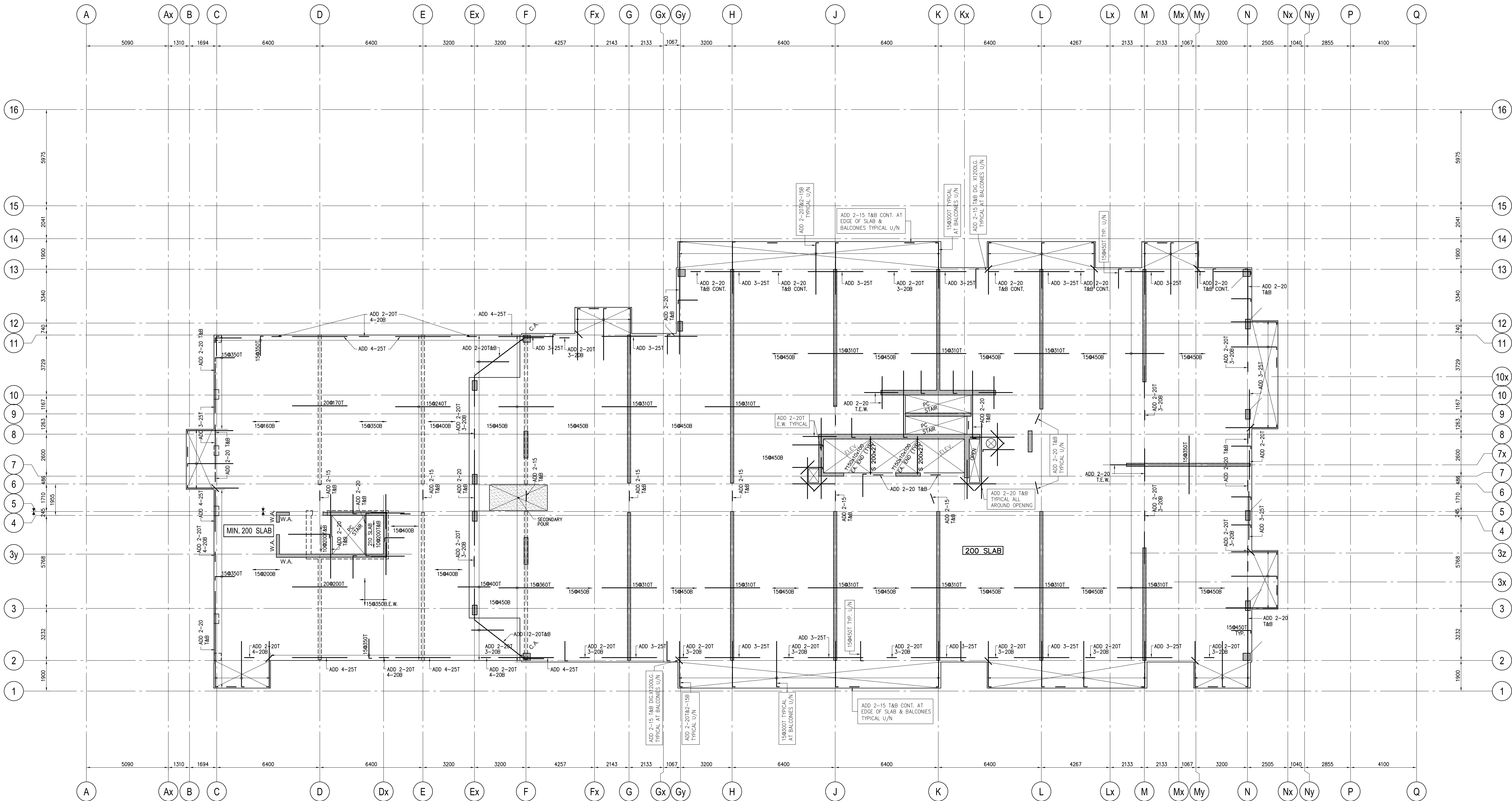




- | | S.I.D. | L.L. |
|---------------------|----------|----------|
| STAIRS & BALCONIES | 0.50 KPa | 4.80 KPa |
| LOCKERS & STORAGE * | 1.30 KPa | 4.80 KPa |
| RESIDENTIAL | 1.3 KPa | 1.9 KPa |
| TOILETS | 1.30 KPa | 2.40 KPa |
| TERRACES | 5.0 KPa | 4.80 KPa |

	S.I.D.	L.L.
STAIRS & BALCONIES	0.50 KPa	4.80 KPa
LOCKERS & STORAGE *	1.30 KPa	4.80 KPa
RESIDENTIAL	1.3 KPa	1.9 KPa
TOILETS	1.30 KPa	2.40 KPa
TERRACES	5.0 KPa	4.80 KPa

S-108



9TH FLOOR FRAMING PLAN

SCALE 1 : 100

- TOP OF SLAB IS AT ELEVATION AS SHOWN ON ARCH. DRAWINGS EXCEPT AS CROSSED AND NOTED ON PLAN.
- CONCRETE STRENGTH AT 28 DAYS SHALL BE:
FOR WALLS AND COLUMNS SEE SCHEDULE
FOR EXT. SLABS 35 MPa
FOR INTERIOR SLABS 25 MPa
FOR BEAMS 35 MPa
CONCRETE EXPOSED TO ELEMENTS SHALL BE 35 MPa WITH 6% TO 8% ENTRAINED AIR.
- FLOOR SLABS ARE DESIGNED FOR FOLLOWING LOADING CONDITIONS :
- MINIMUM YIELD STRESS FOR REINFORCING STEEL SHALL BE 400 MPa.
- TEMPERATURE REINFORCING FOR : 200 SLAB IS 100E250, 210 SLAB IS 100E230.
- NO OPENINGS LARGER THAN 300mm x 300mm ARE ALLOWED IN SLAB OTHER THAN THOSE SHOWN ON DRAWINGS.
- SEE TYPICAL DETAILS ON DRAWINGS S-001 TO S-006.
- SEE GENERAL NOTES ON DRAWING S-001.
- REFER TO ARCH. DRAWINGS FOR SLOPES OF SLAB.
- FOR COLUMN & WALL SCHEDULE SEE DRAWINGS S-301 TO S-306.
- COORDINATE BEAM DEPTH AT DOOR OPENINGS WITH ARCH. DRAWINGS.
- EXTEND TEMP. REINF. TO END OF BALCONIES/OVERHANGS.
- TOP BARS TERMINATING AT EDGE OF SLAB TO HAVE 180° HOOK.

	S.D.	L.L.
STAIRS & BALCONIES	0.50 KPa	4.80 KPa
LOCKERS & STORAGE *	1.30 KPa	4.80 KPa
RESIDENTIAL	1.3 KPa	1.9 KPa
TOILETS	1.30 KPa	2.40 KPa
TERRACES	5.0 KPa	4.80 KPa

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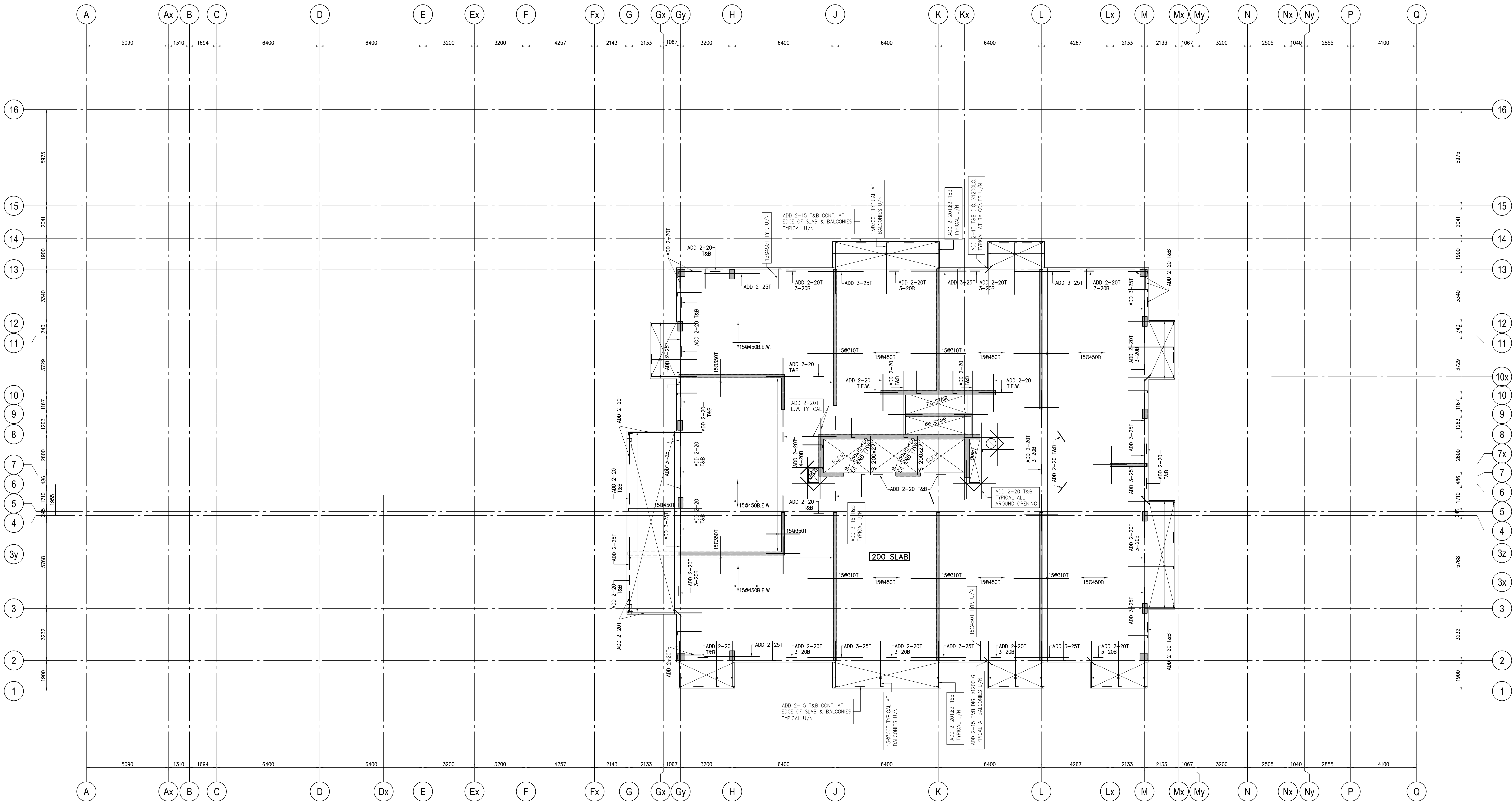
FIRST FLOOR ELEV. 93.60m		
NO.	Issued / Revised	Date
1	ISSUED FOR PERMIT	2014-05-28
2	RE ISSUED FOR PERMIT	2014-07-16
3	ISSUED FOR FORMING TENDER	2014-08-07
4	CONSTRUCTION REVISION	2014-03-11
5	RE ISSUED FOR PERMIT	2014-07-03

ALEXANDRA PARK - BLOCK 11
TORONTO, ONTARIO

project no: 13015
scale: 1:100
drawn by: H.W. HOUMAN
checked by: H.W. HOUMAN
date started: MARCH 2014

9TH FLOOR FRAMING PLAN



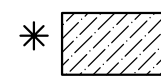


13TH FLOOR FRAMING PLAN

SCALE 1 : 100

- TOP OF SLAB IS AT ELEVATION AS SHOWN ON ARCH. DRAWINGS EXCEPT AS CROSSED AND NOTED ON PLAN.
- CONCRETE STRENGTH AT 28 DAYS SHALL BE:
FOR WALLS AND COLUMNS SEE SCHEDULE
FOR EXT. SLABS 35 MPa
FOR INTERIOR SLABS 25 MPa
FOR BEAMS 35 MPa
CONCRETE EXPOSED TO ELEMENTS SHALL BE 35 MPa WITH 6% TO 8% ENTRAINED AIR.
- FLOOR SLABS ARE DESIGNED FOR FOLLOWING LOADING CONDITIONS :

	S.I.D.	L.L.
STAIRS & BALCONIES	0.50 KPa	4.80 KPa
LOOKERS & STORAGE *	1.30 KPa	4.80 KPa
RESIDENTIAL	1.3 KPa	1.9 KPa
TOILETS	1.30 KPa	2.40 KPa
TERRACES	5.0 KPa	4.80 KPa



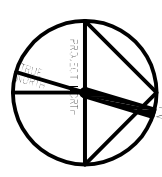
- MINIMUM YIELD STRESS FOR REINFORCING STEEL SHALL BE 400 MPa.
- TEMPERATURE REINFORCING FOR : 200 SLAB IS 108250, 210 SLAB IS 108230.
- NO OPENINGS LARGER THAN 300mm x 300mm ARE ALLOWED IN SLAB OTHER THAN THOSE SHOWN ON DRAWINGS.
- SEE TYPICAL DETAILS ON DRAWINGS S-001 TO S-006.
- SEE GENERAL NOTES ON DRAWING S-001.
- REFER TO ARCH. DRAWINGS FOR SLOPES OF SLAB.
- FOR COLUMN & WALL SCHEDULE SEE DRAWINGS S-301 TO S-306.
- COORDINATE BEAM DEPTH AT DOOR OPENINGS WITH ARCH. DRAWINGS.
- EXTEND TEMP. REINF. TO END OF BALCONIES/OVERHANGS.
- TOP BARS TERMINATING AT EDGE OF SLAB TO HAVE 180° HOOK.

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FIRST FLOOR ELEV. 93.60m

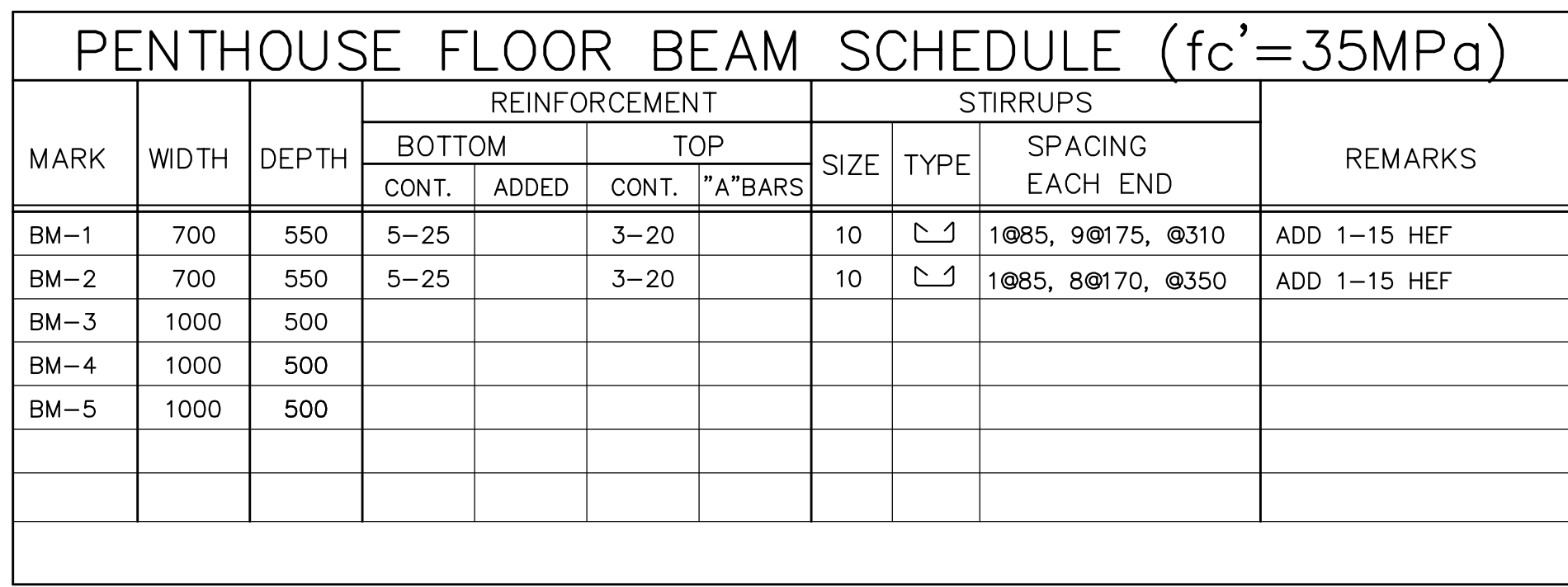
NO.	Issued / Revised	Date
1	ISSUED FOR PERMIT	2014-05-28
2	RE ISSUED FOR PERMIT	2014-07-16
3	ISSUED FOR PERMIT	2014-08-07
4	CONSTRUCTION REVIEW	2014-03-11
5	RE ISSUED FOR PERMIT	2014-07-03



project no:
scale:
drawn by:
reviewed by:
date started:

13015
1:100
H.W.
MARCH 2014

13TH FLOOR
FRAMING PLAN



	S.I.D.	L.L.
STAIRS & BALCONIES	0.50 KPa	4.80 KPa
LOCKERS & STORAGE *	1.30 KPa	4.80 KPa
RESIDENTIAL	1.3 KPa	1.9 KPa
TOILETS	1.30 KPa	2.40 KPa
TERRACES	5.0 KPa	4.80 KPa



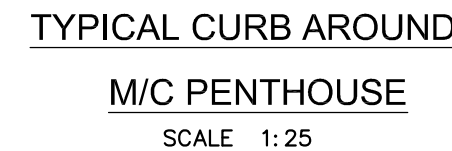
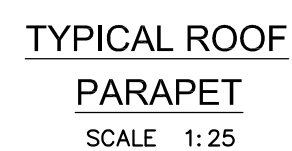
2.7MPa

1.2MPa

2650

SNOW LOAD DRIFT DIAGRAM

SNOW ACCUMULATION ON ROOF



COLUMN & WALL SCHEDULE (BELOW GROUND FLOOR)

[illegible]

COL. No.		Kx.7-Q.7						P.3x-P.7		C 10		D 10		D.10-Q.10			A/Ax.Pd-C.Pd		A/Ax.13-C.13		C.Pd-C.13		P1 Pd		P2 Pd		P3 Pd		P4 Pd		P5 Pd		P6 Pd		P7 Pd		P8 Pd		P8/P9-Pc P8/P9-Pd		P8/P9.Pd-L.Pd		L.Pc-L.Pd		P10 Pd		P11 Pd		P12x Pd		C.15-C.16		COL. No.	
DATA																										DATA																												
GROUND FLOOR																										GROUND FLOOR																												
P1 LEVEL																										P1 LEVEL																												
P2 LEVEL																										P2 LEVEL																												
DATA																										DATA																												
COL. No.																										COL. No.																												

[illegible]

COLUMN AND WALL SCHEDULE NOTES :

1. COLUMN SIZE GIVEN FIRST IN THE SCHEDULE IS IN THE NORTH-SOUTH DIRECTION.
2. COLUMN LOADS ARE GIVEN IN SPECIFIED KILONEWTONS.
3. CONCRETE STRENGTH AT 28 DAYS SHALL BE AS SPECIFIED IN SCHEDULE.
4. MINIMUM YIELD STRESS FOR REINFORCING STEEL SHALL BE 400 MPa.
5. PROVIDE TENSION LAP SPLICE (CLASS 'B') FOR ALL VERTICAL BARS MARKED THUS "▲" IN SCHEDULE.
6. PROVIDE COLUMN TIES IN WALLS THAT HAVE VERTICAL REINFORCING STEEL OF SIZE 20 OR GREATER, FOR ALL THE COLUMNS LOCATED AT THE GROUND FLOOR AND BELOW, PROVIDE 15@200 TIES.
7. FOR ALL WALLS GREATER THAN 2000MM IN LENGTH PROVIDE SHEAR WALL REINFORCING AS PER TABLE 2, FOR ALL OTHER WALLS PROVIDE TEMPERATURE REINFORCING AS PER TABLE 1
8. H.E.F. — DENOTES HORIZONTAL EACH FACE
H.T.B.E.F. — DENOTES HORIZONTAL TOP&BOTTOM EACH FACE
H.T.E.F. — DENOTES HORIZONTAL TOP EACH FACE
V.EAST E. — DENOTES VERTICAL EAST END
V.E. — DENOTES VERTICAL EACH FACE
V.E.E. — DENOTES VERTICAL EACH END
V.N.E. — DENOTES VERTICAL NORTH END
V.S.E. — DENOTES VERTICAL SOUTH END
V.W.E. — DENOTES VERTICAL WEST END
9. REFER TO MECHANICAL DWGS. FOR MECHANICAL WALL OPENINGS.
10. REFER TO ARCHITECTURAL DRAWINGS FOR WALL LENGTHS.
11. REFER TO TYPICAL DETAILS ON DRAWINGS S-001 TO S-004.
12. REFER TO GENERAL NOTES ON DRAWING S-001.
13. UNLESS NOTED OTHERWISE, PROVIDE COLUMN OR WALL DOWELS TO MATCH VERTICAL REINFORCING GIVEN IN SCHEDULE.
14. CONCRETE EXPOSED TO VEHICULAR TRAFFIC SHALL HAVE 6% TO 8% ENTRAINED AIR AND SHALL BE PROPORTIONED FOR C-1 EXPOSURE CONDITIONS.

COLUMN, WALL & FOOTING DOWEL SCHEDULE										
LAP SPlice IN WALL OR COLUMN AND DEVELOPMENT LENGTH IN CAP OR FOOTING										
DATA		BAR SIZE	10	15	20	25	30	35		
F _y = 400 F _u = 250	TENSION	LAP SPlice	490	740	980	1530	1830	2130		
		DEVELOPMENT LENGTH	380	570	750	1170	1410	1640		
	COMPRESSION	LAP SPlice	300	440	590	730	880	1030		
		DEVELOPMENT LENGTH	220	310	370	480	570	690		
F _y = 400 F _u = 300	TENSION	LAP SPlice	450	670	890	1390	1670	1950		
		DEVELOPMENT LENGTH	350	520	690	1070	1290	1500		
	COMPRESSION	LAP SPlice	300	440	590	730	880	1030		
		DEVELOPMENT LENGTH	200	280	340	440	530	630		
F _y = 400 F _u = 350	TENSION	LAP SPlice	420	620	830	1290	1550	1800		
		DEVELOPMENT LENGTH	320	480	640	990	1190	1390		
	COMPRESSION	LAP SPlice	300	440	590	730	880	1030		
		DEVELOPMENT LENGTH	200	280	340	440	530	630		
F _y = 400 F _u = 400	TENSION	LAP SPlice	390	580	770	1210	1450	1690		
		DEVELOPMENT LENGTH	300	450	600	930	1110	1300		
	COMPRESSION	LAP SPlice	300	440	590	730	880	1030		
		DEVELOPMENT LENGTH	200	280	340	440	530	630		

WALL THICKNES	DIRECTION	WALL EXPOSED OR ENCLOSED BOTH SIDES	WALL EXPOSED ONE SIDE	
			EXPOSED SIDE	ENCLOSED SIDE
150mm	HORIZ.	10 @ 330 (1 LAYER)	10 @ 330 (1 LAYER)	
	VERT.	10 @ 450 (1 LAYER)	10 @ 450 (1 LAYER)	
200mm	HORIZ.	10 @ 450 EA. FACE	10 @ 420	10 @ 450
	VERT.	10 @ 450 EA. FACE	10 @ 450	10 @ 450
250mm	HORIZ.	10 @ 400 EA. FACE	10 @ 330	10 @ 450
	VERT.	10 @ 450 EA. FACE	10 @ 450	10 @ 450
300mm	HORIZ.	10 @ 330 EA. FACE	10 @ 250	10 @ 450
	VERT.	10 @ 440 EA. FACE	10 @ 400	10 @ 450
350mm	HORIZ.	10 @ 280 EA. FACE	10 @ 210	10 @ 430
	VERT.	10 @ 380 EA. FACE	10 @ 300	10 @ 450
400mm	HORIZ.	10 @ 250 EA. FACE	15 @ 380	10 @ 370
	VERT.	10 @ 330 EA. FACE	10 @ 250	10 @ 450

NOTE: SIZE AND SPACING OF DOWELS TO BE SAME AS VERTICAL REINF. IN WALL

		SHEAR WALL REINFORCING		
WALL THICKNESS	DIRECTION	WALL EXPOSED OR ENCLOSED BOTH SIDES	WALL EXPOSED ONE SIDE	
			EXPOSED SIDE	ENCLOSED SIDE
150mm	HORIZ.	10 @ 270 (1 LAYER)	10 @ 270 (1 LAYER)	
	VERT.	10 @ 270 (1 LAYER)	10 @ 270 (1 LAYER)	
200mm	HORIZ.	10 @ 400 EA. FACE	10 @ 300	10 @ 450
	VERT.	10 @ 400 EA. FACE	10 @ 300	10 @ 450
250mm	HORIZ.	10 @ 320 EA. FACE	10 @ 240	10 @ 450
	VERT.	10 @ 320 EA. FACE	10 @ 240	10 @ 450
300mm	HORIZ.	10 @ 270 EA. FACE	10 @ 200	10 @ 400
	VERT.	10 @ 270 EA. FACE	10 @ 200	10 @ 400
350mm	HORIZ.	15 @ 450 EA. FACE	15 @ 340	15 @ 450
	VERT.	15 @ 450 EA. FACE	15 @ 340	15 @ 450
400mm	HORIZ.	15 @ 400 EA. FACE	15 @ 300	15 @ 450
	VERT.	15 @ 400 EA. FACE	15 @ 300	15 @ 450
450mm	HORIZ.	15 @ 350 EA. FACE	15 @ 260	15 @ 450
	VERT.	15 @ 350 EA. FACE	15 @ 260	15 @ 450
NOTE: SIZE AND SPACING OF DOWELS TO BE SAME AS VERTICAL REINF. IN WALL				

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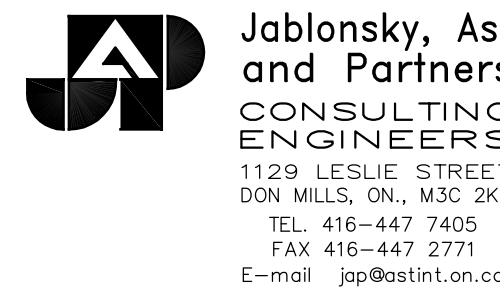
COLUMN & WALL SCHEDULE

COL. No. DATA	f'c	A 2	A 3	A 3y		A.4-B.6	A 7	A 9		Ax 2	Ax 3	Ax 7-8	Ax 9		C 2	C 3		C 5		C 6	C 10		C 11		D.2-D.3y	D.3y-D.5	D.3y-Dx.3y	D.5-Dx.5	Dx.3y-Dx.4		E.2-E.5		D.6-D.11	E.6-E.11		COL. No. DATA	
UPPER ROOF																																					UPPER ROOF
PENTHOUSE	4350																																				PENTHOUSE
14TH FLOOR	3470																																				14TH FLOOR
13TH FLOOR	3250																																				13TH FLOOR
12TH FLOOR	2950																																				12TH FLOOR
11TH FLOOR	3050																																				11TH FLOOR
10TH FLOOR	2950																																				10TH FLOOR
9TH FLOOR	3050	f'c=10 MPa																								200 WALL	300 WALL	300 WALL	200 WALL	200 WALL							9TH FLOOR
8TH FLOOR	3250														450x450 8-20	300x600 6-20		300x600 6-20		300x600 6-20		450x450 8-20				ADD 4-20 VEE ▲	ADD 4-20 VEE ▲	ADD 4-20 VEE ▲	ADD 4-20 VEE ▲	ADD 4-20 VEE ▲							8TH FLOOR
7TH FLOOR	2950																									300 WALL	300 WALL	300 WALL	300 WALL	300 WALL							7TH FLOOR
6TH FLOOR	2950																									300 WALL	300 WALL	300 WALL	300 WALL	300 WALL							6TH FLOOR
5TH FLOOR	3050														200 WALL 100x300 HEF 100x400 VEF	ADD 2-20H T&B ADD 4-20 VEE ▲		300x600 6-20		300x600 6-20					200 WALL	300 WALL 100x300 HEF 100x70 VEF	300 WALL	300 WALL	300 WALL	300 WALL	300 WALL	200 WALL					5TH FLOOR
4TH FLOOR	2790		450x450 8-20	300x600 6-20	300x600 6-20	200 WALL	300x600 6-20	300x600 6-20	600x250 8-20		600x300 6-20	600x300 6-20	600x300 6-20		450x450 8-20	200 WALL	BEAM	200 WALL		200 WALL						ADD 4-20 VEE ▲	300 WALL	300 WALL	300 WALL	300 WALL	300 WALL	200 WALL					4TH FLOOR
3RD FLOOR	2790		450x450 8-20	300x600 6-20	300x600 6-20	200 WALL									200 WALL	200 WALL	200 WALL	200 WALL	200 WALL	200 WALL	200 WALL	200 WALL	200 WALL	200 WALL	200 WALL	200 WALL	200 WALL	200 WALL	200 WALL	200 WALL	200 WALL	200 WALL	200 WALL	200 WALL	200 WALL	3RD FLOOR	
2ND FLOOR	2950		250 WALL 100x200 HEF 100x400 VEF	BEAM		200 WALL 100x200 HEF 100x400 VEF	300x600 6-20	300x600 6-20	600x250 8-20	BEAM	600x300 6-20	600x300 6-20	600x300 6-20		200 WALL 100x200 HEF 100x300 VEF	200 WALL 100x200 HEF 100x300 VEF	200 WALL 100x200 HEF 100x300 VEF	200 WALL 100x200 HEF 100x300 VEF	200 WALL 100x200 HEF 100x300 VEF	200 WALL 100x200 HEF 100x300 VEF	200 WALL 100x200 HEF 100x300 VEF	200 WALL 100x200 HEF 100x300 VEF	200 WALL 100x200 HEF 100x300 VEF	200 WALL 100x200 HEF 100x300 VEF	200 WALL 100x200 HEF 100x300 VEF	200 WALL 100x200 HEF 100x300 VEF	200 WALL 100x200 HEF 100x300 VEF	200 WALL 100x200 HEF 100x300 VEF	200 WALL 100x200 HEF 100x300 VEF	200 WALL 100x200 HEF 100x300 VEF	200 WALL 100x200 HEF 100x300 VEF	200 WALL 100x200 HEF 100x300 VEF	200 WALL 100x200 HEF 100x300 VEF	200 WALL 100x200 HEF 100x300 VEF	2ND FLOOR		
GROUND	4250	f'c=15 MPa	250 WALL 150x300 HEF 100x200 VEF	BEAM		250 WALL 150x300 HEF 100x200 VEF	BEAM	300x600 6-20	BEAM	300x700 8-20	300x700 8-20	300x600 6-20		300x250 8-20	ADD 4-20H T&B ADD 4-25 VEE ▲	700x300 8-20	250 WALL 150x300 HEF 100x200 VEF	BEAM	250 WALL 150x300 HEF 100x200 VEF	BEAM	300 WALL 150x300 HEF 100x200 VEF	SLAB	SLAB	SLAB	250 WALL 150x300 HEF 100x200 VEF	BEAM	250 WALL 150x300 HEF 100x200 VEF	BEAM	250 WALL 150x300 HEF 100x200 VEF	BEAM	250 WALL 150x300 HEF 100x200 VEF	BEAM	250 WALL 150x300 HEF 100x200 VEF	BEAM	250 WALL 150x300 HEF 100x200 VEF	GROUND	
EL. 0.000			ADD 4-20H T&B ADD 4-20 VEE ▲	300x600 6-20	SLAB	ADD 4-20H T&B ADD 4-20 VEE ▲	300x600 6-20	300x700 8-20	300x700 8-20	300x600 6-20	300x250 8-20	ADD 4-20H T&B ADD 4-25 VEE ▲	700x300 8-20	SLAB	ADD 4-20H T&B ADD 4-25 VEE ▲	SLAB	SLAB	SLAB	SLAB	SLAB	SLAB	SLAB	SLAB	SLAB	SLAB	SLAB	SLAB	SLAB	SLAB	SLAB	SLAB	SLAB	SLAB	SLAB	SLAB	EL. 114.75	
DATA COL. No.	f'c	A 2-A.3	A 3y			Ax.4-B.8	A 6	A 8	A 9		Ax 2	Ax-3	Ax 7-8	Ax 9		C.2-C.3y				C.5-Cx.5					D 2	D.3y-D.5	D.3y-Dx.3y	D.5-Dx.5	C-3y/4 X F-3y/4		E 2		D.9-D.13	C.8-Ex.8	E.9-E.13	DATA COL. No.	

[illegible]

COLUMN & WALL SCHEDULE

COL. No. DATA	Fc	Ex 3	Ex 4	Ex 7	Ex 10	F 2	F.3y-G.3y	G 2	G 3	F.8-G.8	G 8	G 10	G 11	Gy 12	Gy 13	Gy 2	Gy 5	Gy 8	H 2	G-3y, H/G-5	Gy-10/11 H/J-9	H 13	J.9-J.13	J/H.10-J/H.14	J/K.10-J/K.14	COL. No. DATA				
UPPER ROOF																										UPPER ROOF				
PENTHOUSE																										PENTHOUSE				
14TH FLOOR														300x600 6-20				300x600 6-20	300x600 6-20	300x600 6-20	200 WALL ADD 2-20 VEE ▲	200 WALL ADD 2-20 VEE ▲		200 WALL ADD 4-20 VEE ▲	200 WALL ADD 4-20 VEE ▲		14TH FLOOR			
13TH FLOOR																					200 WALL ADD 2-20 VEE ▲	200 WALL ADD 2-20 VEE ▲		200 WALL ADD 4-20 VEE ▲	200 WALL ADD 4-20 VEE ▲		13TH FLOOR			
12TH FLOOR																					200 WALL ADD 2-20 VEE ▲	200 WALL ADD 2-20 VEE ▲					12TH FLOOR			
11TH FLOOR							200 WALL 450x450 8-20														200 WALL ADD 2-20 VEE ▲	200 WALL ADD 2-20 VEE ▲					11TH FLOOR			
10TH FLOOR							200 WALL														200 WALL ADD 2-20 VEE ▲	200 WALL ADD 2-20 VEE ▲					10TH FLOOR			
9TH FLOOR							250 WALL ADD 4-20 VEE ▲														200 WALL ADD 4-20 VEE ▲	200 WALL ADD 4-20 VEE ▲					9TH FLOOR			
8TH FLOOR							ADD 4-20 VEE ▲														ADD 4-20 VEE ▲	ADD 4-20 VEE ▲					8TH FLOOR			
7TH FLOOR							ADD 4-20 VEE ▲														ADD 4-20 VEE ▲	ADD 4-20 VEE ▲					7TH FLOOR			
6TH FLOOR																					200 WALL ADD 4-20 VEE ▲	200 WALL ADD 4-20 VEE ▲					6TH FLOOR			
5TH FLOOR							200 WALL														200 WALL ADD 4-20 VEE ▲	200 WALL ADD 4-20 VEE ▲					5TH FLOOR			
4TH FLOOR							ADD 4-20 VEE ▲														ADD 4-20 VEE ▲	ADD 4-20 VEE ▲					4TH FLOOR			
3RD FLOOR							200 WALL 158300 HEF 108300 VEF ADD 4-25H TOP ADD 4-25 VEE ▲														200 WALL 158300 HEF 108300 VEF ADD 4-25H TOP ADD 4-25 VEE ▲	200 WALL 158300 HEF 108300 VEF ADD 4-25H TOP ADD 4-25 VEE ▲		200 WALL ADD 4-25 VEE ▲	200 WALL ADD 4-25 VEE ▲		3RD FLOOR			
2ND FLOOR							200 WALL 158300 HEF 108300 VEF ADD 4-25H BOT ADD 4-25 VEE ▲														200 WALL 158300 HEF 108300 VEF ADD 4-25H BOT ADD 4-25 VEE ▲	200 WALL 158300 HEF 108300 VEF ADD 4-25H BOT ADD 4-25 VEE ▲		200 WALL ADD 4-25 VEE ▲	200 WALL 108200 HEF 108300 VEF ADD 4-25 VEE ▲		2ND FLOOR			
GROUND							250 WALL 158300 HEF 108200 VEF ADD 4-20H TAB ADD 4-25 VEE ▲														250 WALL 158300 HEF 108200 VEF ADD 4-25 VEE ▲	250 WALL 158300 HEF 108200 VEF ADD 4-25 VEE ▲		250 WALL 158300 HEF 108200 VEF ADD 4-25 VEE ▲	250 WALL 158300 HEF 108200 VEF ADD 4-25 VEE ▲		GROUND			
EL. 0.000							SLAB														SLAB	SLAB					EL. 114.75			
DATA COL. No.	Fc	Ex 3	Ex.3y-Ex.8	Ex 10	F 2	F.2-F.5	G 2	G.2-G.5	G 3y	Ex.8-F.8	F.9-F.13	Fx 4	Fx 10	Fx 13	Fx 10	Gy 12	Gy 13	Gy 2	Gy 5	Gy 8	H 2	H 3-3y	H 7-8	H 10	H 13	J.1-J.4	J.10x-J.14	J/H.10-J/H.14	J/K.10-J/K.14	DATA COL. No.



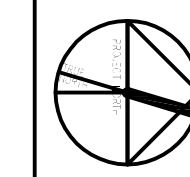
FIRST FLOOR ELEV. 93.60

no.	issued / revised	date:
1	ISSUED FOR PERMIT	2014-05
2	RE ISSUED FOR PERMIT	2014-07
3	ISSUED FOR FORMING TENDER	2014-08
4	CONSTRUCTION REVIEW	2014-12
5	RE ISSUED FOR PERMIT	2015-07

11

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project no:	13015
scale:	AS NOTED
drawn by:	H.W.
reviewed by:	HOUMAN
date started:	MARCH 20

COLUMN & WALL SCHEDULE

COLUMN & WALL SCHEDULE

[illegible]

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COLUMN & WALL SCHEDULE

COL. No. DATA	f _c	M 2	M 3	M 4	M 9		Lx.6-Lx.13	M 12	M 13	Mx.6-Mx.13							N 2	N 3	N 5		N 10		N 12	N 13					N.3-Nx.3		Q 3							Ny 3z		COL. No. DATA
UPPER ROOF																																								UPPER ROOF
PENTHOUSE																																								PENTHOUSE
14TH FLOOR			450x450 8-20		300x600 6-20			300x600 6-20		450x450 8-20							450x450 8-20	300x600 6-20	300x600 6-20	300x600 6-20		300x600 6-20		300x600 6-20	450x450 8-20															14TH FLOOR
13TH FLOOR																																								13TH FLOOR
12TH FLOOR																																								12TH FLOOR
11TH FLOOR																																								11TH FLOOR
10TH FLOOR			450x450 8-20		300x600 6-20			300x600 6-20		450x450 8-20												300x600 6-20		300x600 6-20	450x450 8-20															10TH FLOOR
9TH FLOOR	f _c =30 MPa		M.2-M.3z 200 WALL		BEAM			M.10x-M.13 200 WALL																																9TH FLOOR
8TH FLOOR			ADD 4-20 VEE ▲					ADD 4-20 VEE ▲																															8TH FLOOR	
7TH FLOOR																																								7TH FLOOR
6TH FLOOR																	450x450 8-20	300x600 6-20	300x600 6-20												250 WALL					300x600 6-20	Nx.3z-Q.3z		6TH FLOOR	
5TH FLOOR							SLAB 200 WALL	SLAB 200 WALL		SLAB 200 WALL			300 WALL ADD 4-20 VEE ▲	300 WALL ADD 4-20 VEE ▲			200 WALL ADD 4-20 VEE ▲						900x250 8-20							250 WALL ADD 4-20 VEE ▲		250x600 6-20					200 WALL ADD 4-20 VEE ▲		5TH FLOOR	
4TH FLOOR			200 WALL ADD 4-20 VEE ▲				200 WALL 158300 HEF 108300 VEF ADD 4-25H TOP ADD 4-20 VEE ▲	200 WALL 158300 HEF 108300 VEF ADD 4-25H TOP ADD 4-20 VEE ▲		200 WALL 158300 HEF 108300 VEF ADD 4-25H TOP ADD 4-20 VEE ▲			300 WALL ADD 4-20 VEE ▲	300 WALL ADD 4-20 VEE ▲			200 WALL 158200 HEF 108300 VEF ADD 4-20 VEE ▲														250 WALL ADD 4-20 VEE ▲						200 WALL 108200 HEF 108200 VEF ADD 4-20H TOP ADD 4-25 VEE ▲		4TH FLOOR	
3RD FLOOR			ADD 4-20 VEE ▲				200 WALL 158300 HEF 108300 VEF ADD 4-25H BOT ADD 4-25 VEE ▲	200 WALL 158300 HEF 108300 VEF ADD 4-25H BOT ADD 4-25 VEE ▲		200 WALL 158300 HEF 108300 VEF ADD 4-25H BOT ADD 4-25 VEE ▲			300 WALL ADD 4-25 VEE ▲	300 WALL ADD 4-25 VEE ▲			200 WALL 158200 HEF 108300 VEF ADD 4-25H TOP ADD 4-25 VEE ▲														250 WALL ADD 4-25 VEE ▲						200 WALL 108200 HEF 108200 VEF ADD 4-20H TOP ADD 4-25 VEE ▲		3RD FLOOR	
2ND FLOOR			200 WALL 158300 HEF 108300 VEF ADD 4-25H TAB ADD 4-25 VEE ▲		SLAB		SLAB	300x600 10-30		800x300 10-30		300 WALL 108200 HEF 108200 VEF ADD 4-20H TOP ADD 4-25 VEE ▲	300 WALL 108200 HEF 108200 VEF ADD 4-25 VEE ▲			200 WALL 158200 HEF 108300 VEF ADD 4-25H BOT ADD 4-25 VEE ▲						900x250 8-20		250 WALL 158300 HEF 108200 VEF ADD 4-25 VEE ▲					250 WALL 158300 HEF 108200 VEF ADD 4-25H TAB ADD 4-25 VEE ▲		250x600 6-20				300 WALL 108200 HEF 108200 VEF ADD 4-25 VEE ▲		200 WALL 108200 HEF 108200 VEF ADD 4-20H BOT ADD 4-25 VEE ▲		2ND FLOOR	
GROUND	f _c =35 MPa		250 WALL 158300 HEF 108200 VEF ADD 4-25 H TOP TAB ADD 4-20 VEE ▲		SLAB							300 WALL 158200 HEF 108200 VEF ADD 4-25 H TOP TAB ADD 4-25 VEE ▲	300 WALL 158300 HEF 108200 VEF ADD 4-25 H TOP TAB ADD 4-25 VEE ▲																250 WALL 158300 HEF 108200 VEF ADD 4-20H TOP TAB ADD 4-25 VEE ▲							300 WALL 108200 HEF 108200 VEF ADD 4-25 VEE ▲		250 WALL 158300 HEF 108200 VEF ADD 4-20H TOP TAB ADD 4-25 VEE ▲		GROUND
EL. 0.000			BEAM																																				EL. 114.75	
DATA COL. No.	f _c	M.1-M.3x					Lx 13	M.10x-M.13	Mx 13		M.12/13-M.14	My.12-My.14	My.3x-My.5	My.3x-N.3x	My.5-N.5	N 1/2	N.2-N.5							N.11-N.13	Ny 2	P.1-P.3			N.3-Q.3				L.3x-Q.3x	L/LX.7-Q.7		Ny.3x-Ny.3y		Q 3z COL. No.		

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FIRST FLOOR ELEV. 93.60m		
No.	Issued / revised	Date
1	ISSUED FOR PERMIT	2014-05-01
2	REVISED FOR PERMIT	2014-05-01
3	REVISED FOR PERMIT TENDON	2014-05-07
4	COMPLETED FOR PERMIT	2014-05-07
5	REVISED FOR PERMIT	2014-05-07
6		
7		
8		
9		
10		

ALEXANDRA PARK - BLOCK 11
TORONTO, ONTARIO

project no:
scale:
drawn by:
reviewed by:
date started:

13015
AS NOTED
H.W.
HOLMAN
MARCH 2014

COLUMN & WALL SCHEDULE

[illegible]

no.	Issued / revised	date:
1	ISSUED FOR PERMIT	2014-05-30
2	RE ISSUED FOR PERMIT	2014-07-10
3	ISSUED FOR FORMING TENDER	2014-09-27
4	CONSTRUCTION REVIEW	2014-10-15
5	RE ISSUED FOR PERMIT	2015-01-03

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ARK - BLOC

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ALEXANDER
 TORONTO, ONTARIO



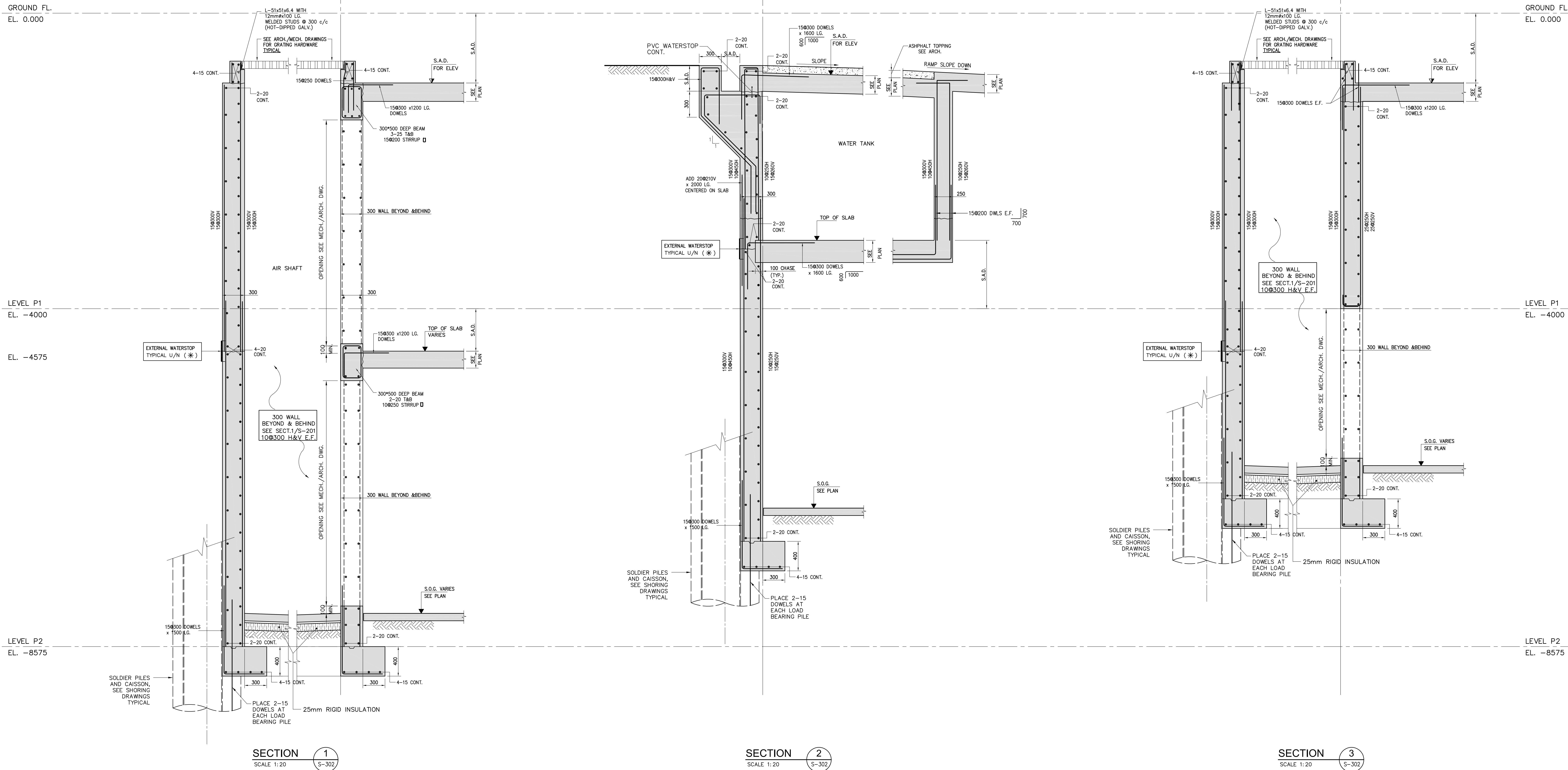
ALBION
TORONTO

Project no: 13015

Case:	AS NOTED
Drawn by:	H.W.
Reviewed by:	HOUMAN
Date started:	MARCH 2014

COLUMN & WALL
SCHEDULE

S-206



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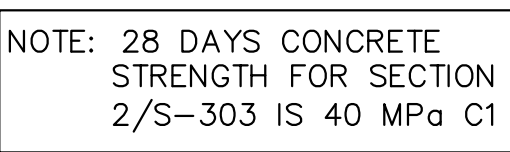
FIRST FLOOR ELEV. 93.60m		
NO.	Issued / Revised	Date
1	ISSUED FOR PERMIT	2014-05-28
2	RE ISSUED FOR PERMIT	2014-07-16
3	ISSUED FOR PERMIT	2014-08-07
4	CONSTRUCTION REVIEW	2014-09-11
5	RE ISSUED FOR PERMIT	2014-09-23
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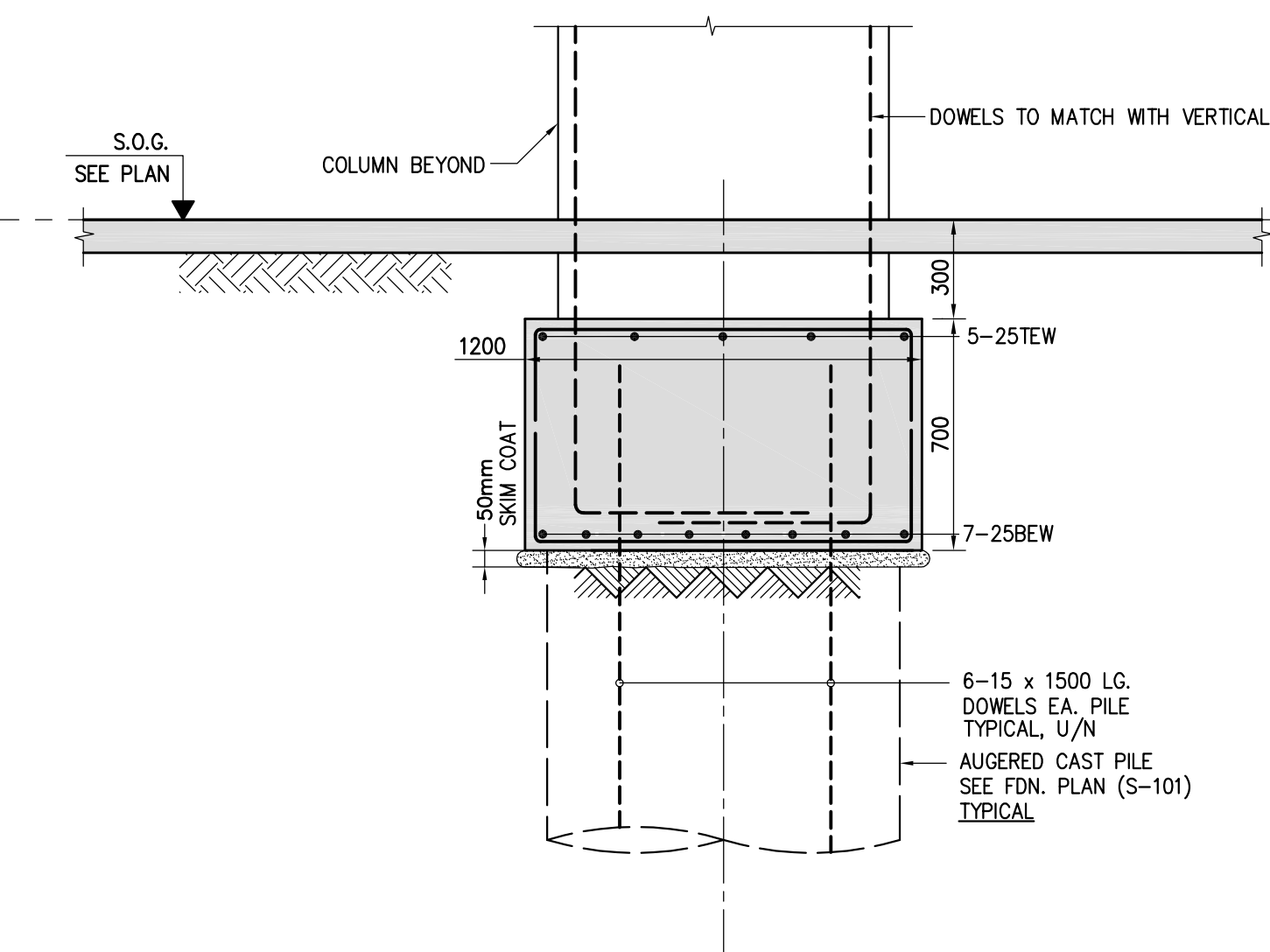
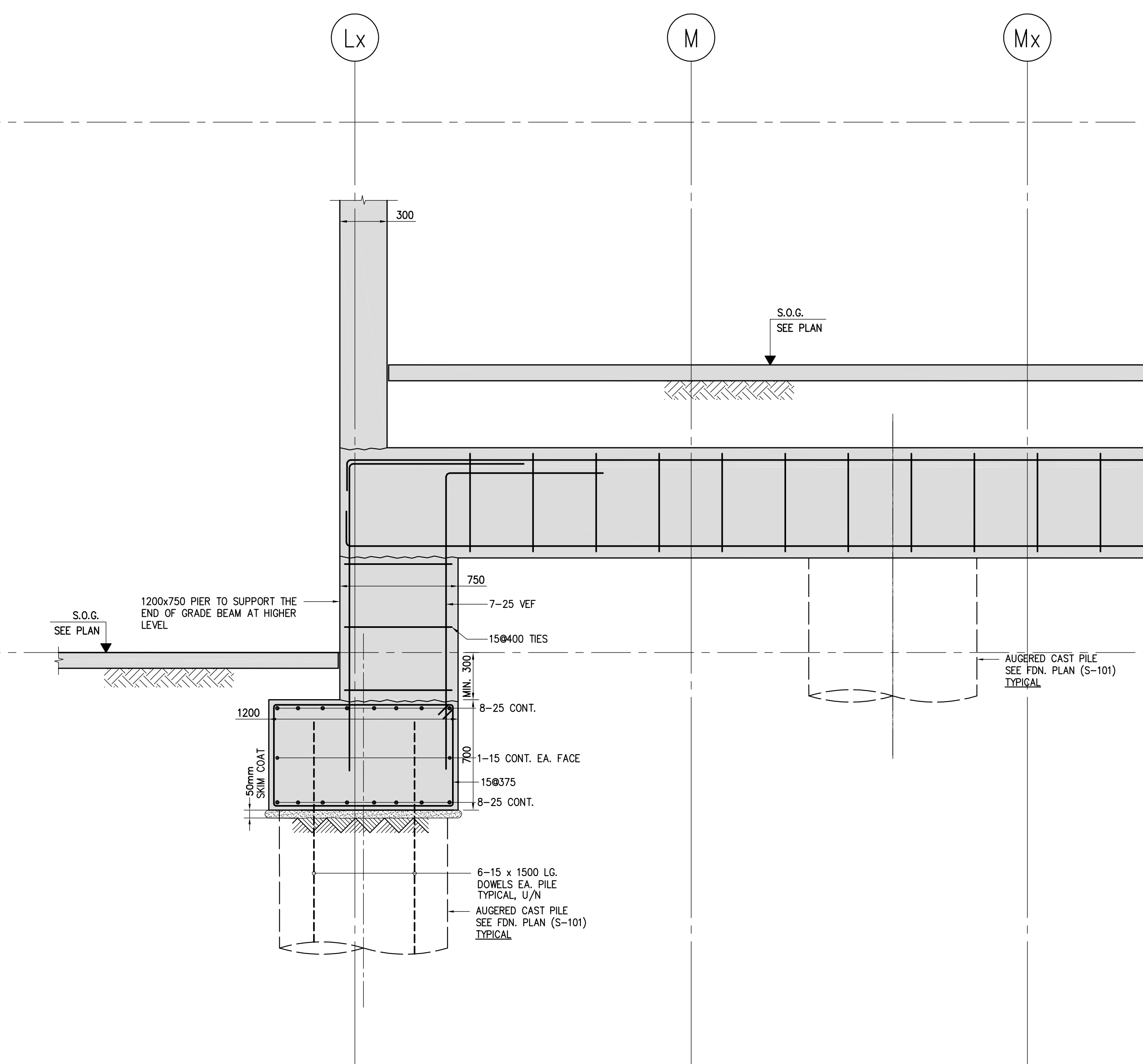
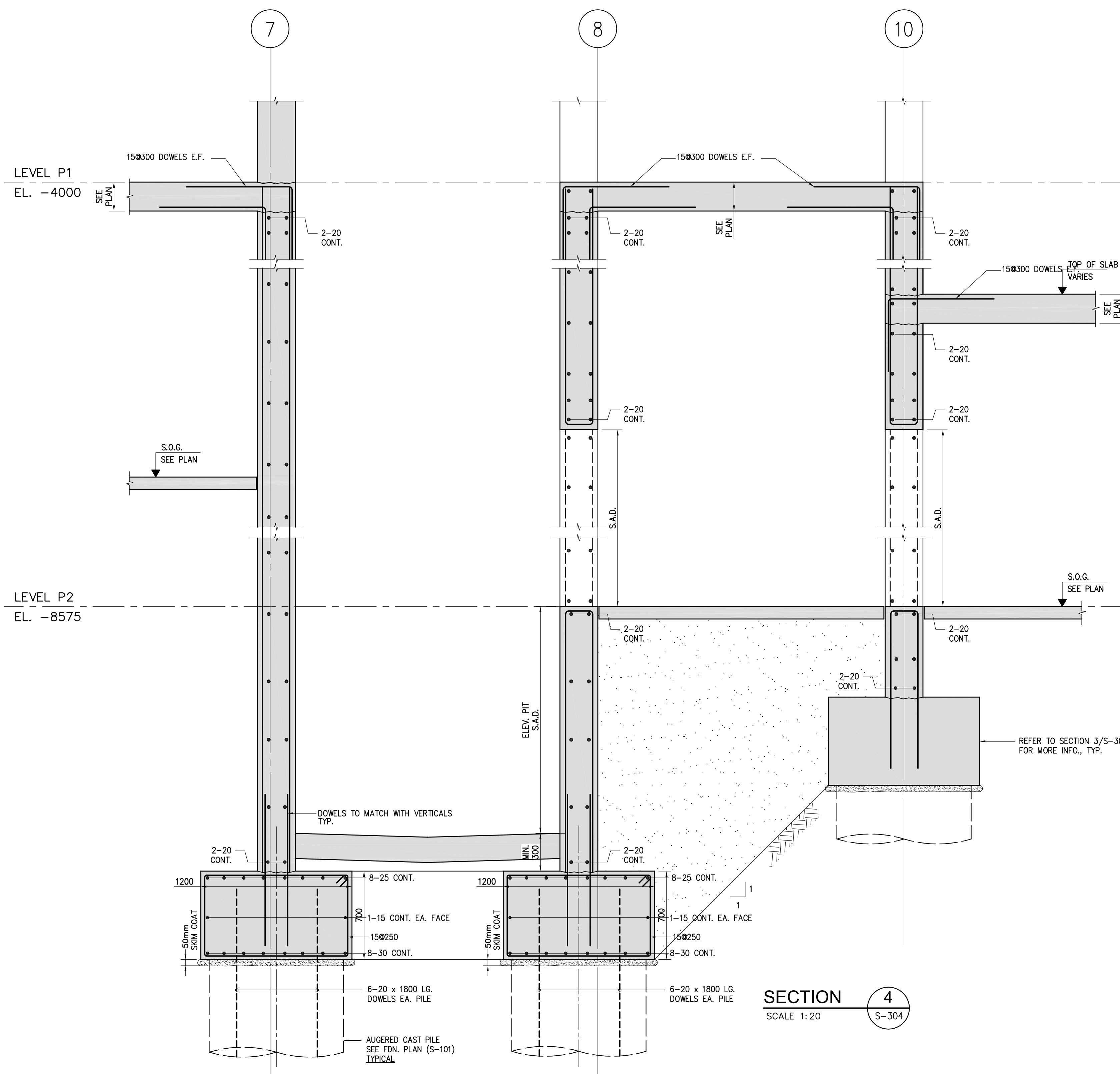
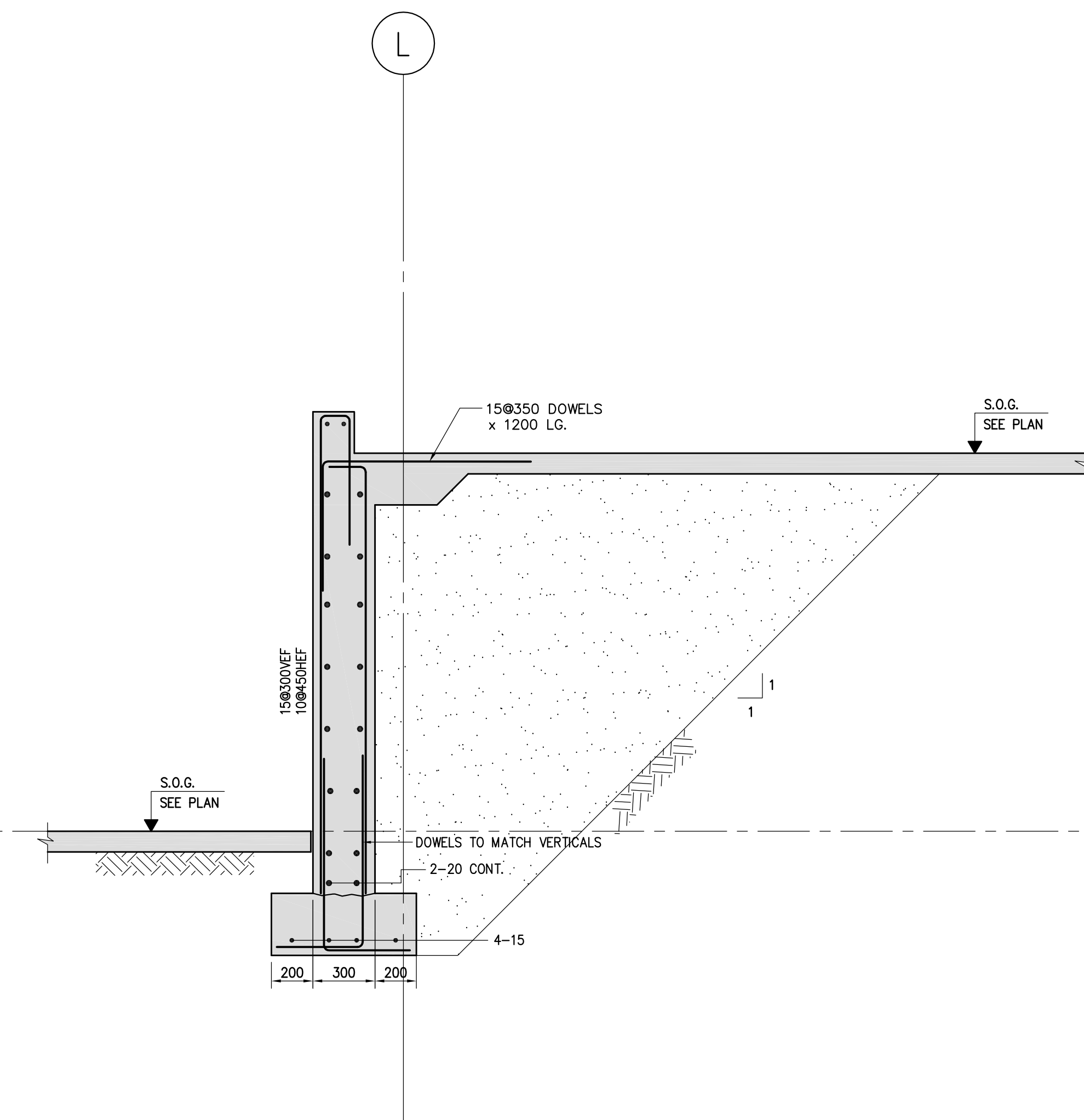
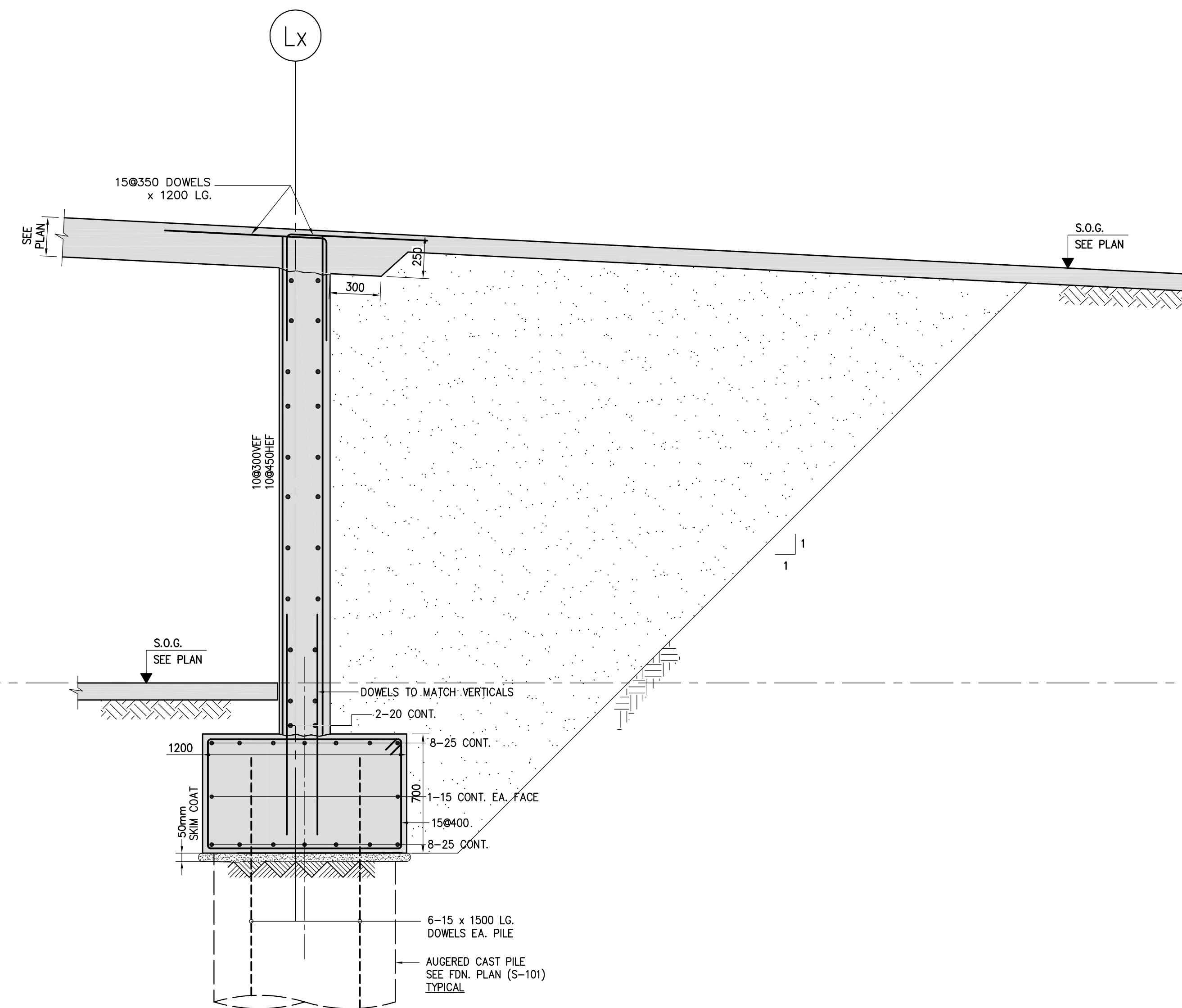
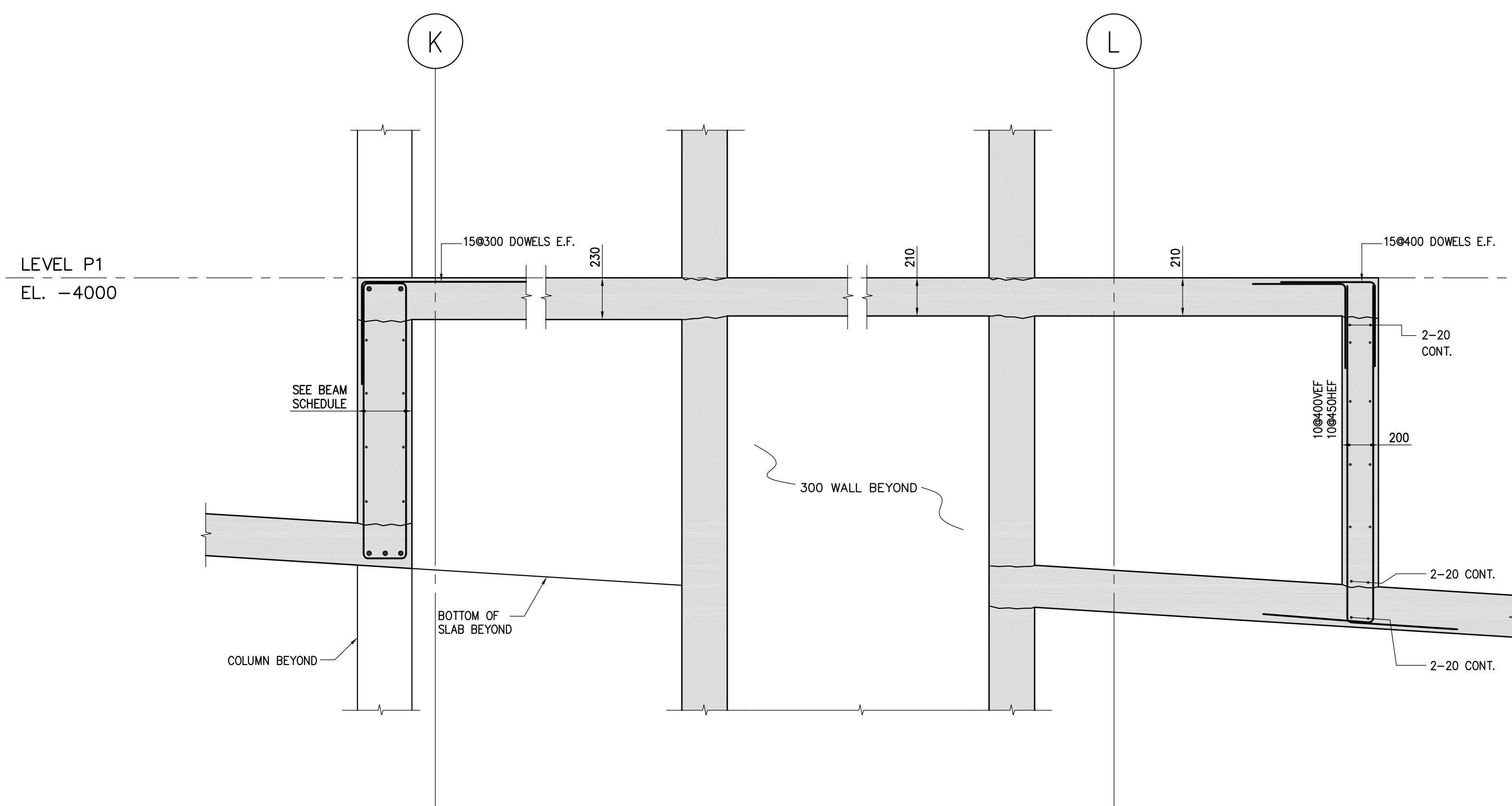
ALEXANDRA PARK - BLOCK 11
TORONTO, ONTARIO

project no: 13015
scale: 1:20
drawn by: H.W.
reviewed by: H.W.
date started: MARCH 2014

FOUNDATION SECTION

S-302





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FIRST FLOOR ELEV. 93.60m		
NO.	Issued / Revised	Date
1	ISSUED FOR PERMIT	2014-05-28
2	RE ISSUED FOR PERMIT	2014-07-16
3	ISSUED FOR FORMING TRENCH	2014-08-07
4	CONSTRUCTION REVIEW	2014-03-11
5	RE ISSUED FOR PERMIT	2015-07-03

ALEXANDRA PARK - BLOCK 11
TORONTO, ONTARIO

project no: 13015
scale: 1:25
drawn by: H.W. HOUZMAN
reviewed by:
date started: MARCH 2014

FOUNDATION SECTION

[illegible]

project no:	13015
calc:	1:25
drawn by:	H.W.
reviewed by:	HOUMAN
date started:	MARCH 2014